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PROPOSED CONSTRUCTION OF RESIDENTIAL GROUP DEVELOPMENT COMPRISING 4 BLOCKS AND COMBINED BASEMENT; BLOCK 1: BASEMENT FLOOR + GROUND FLOOR + 5 FLOORS (18.15M HEIGHT) RESIDENTIAL BUILDING WITH 95 DWELLING UNITS, BLOCK 2: BASEMENT FLOOR + GROUND FLOOR + 4 FLOORS + 5TH FLOOR PART (18.15M HEIGHT) RESIDENTIAL BUILDING WITH 54 DWELLING UNITS AND SWIMMING POOL, INDOOR GAMES ROOM, ASSOCIATION ROOM, VIDEO GAMES ROOM & KIDS PLAY ROOM; BLOCK 3: BASEMENT FLOOR + GROUND FLOOR + 5 FLOORS (18.15M HEIGHT) RESIDENTIAL BUILDING WITH 36 DWELLING UNITS; BLOCK 4: BASEMENT FLOOR + GROUND FLOOR + 5 FLOORS (18.15M HEIGHT) RESIDENTIAL BUILDING WITH 24 DWELLING UNITS; TOTALLY 209 DWELLING UNITS AVAILING PREMIUM FSI IN S.NOS. 10/1, 10/5, 10/6, 10/7, 10/8, 10/11A, 11/7A, 11/8A1A, 11/8B, 11/8C1A, 11/8C1B, 11/8C2, 11/8C3, 11/8C4, 11/9A, 11/10B1B, 11/10B2, 12/1A, AND 12/1B OF NEELANKARAI VILLAGE AND S.NO.23/24 OF SEEVARAM VILLAGE WITHIN THE LIMIT OF GREATER CHENNAI CORPORATION.

TYPICAL STRUCTURAL DESIGN CALCULATION

1 General:

All residential buildings are Basement + Ground + 5 upper floors. Basement floor is used as car parking purpose. Upper floors are for Residential use. One typical block design is given

2 Design Basis:

Latest Design codes of BIS-456, 875 and 1893 are followed with SP-16 Design Aid book. A typical 2-D Frame was taken for analysis to arrive at the bending moment for column design. Since the building is 6 stories, wind force is taken into account. Also a live load reduction factor of 0.2 is taken for foundation and column design.

Pile foundation is proposed as per as per soil report.

Fe- 500 steel is used as main steel and minimum M25 concrete is provided

3 Earthquake consideration:

For better stability during earthquake, the columns are braced in both direction with beams. Continuous lintels are specified for outer walls and column ties are closely spaced for better ductility. For building on basement and stilt, minimum size is taken as 300 mm(12")


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EARTHQUAKE LOAD

Design horizontal Seismic coeff. $A_g = \frac{Z}{2} \times \frac{I}{R} \times \frac{S_a}{g}$

Z = Zone Factor = 0.16 for Chennai

I = Importance Factor = 1.0

R = Response reduction Factor = 5

$\frac{S_a}{g}$ = Avg. response
acc. coeff. Depends on T_a

T_a = Fundamental natural period

= $\frac{0.09 h}{\sqrt{d}}$ for infill walls
 h = height of the building
 d = Width of the building
= 0.075 h for office building

W = seismic weight of building
= 100% D.L + 25% L.L for residential ≤ 300
= 100% D.L + 50% L.L for residential > 300

Base Seismic Shear $V_b = A_g \times W$

4

WIND LOAD

$V_z = V_b \times K_1 \times K_2 \times K_3$

V_b = 50 m/sec in Chennai

K_1 = Probability Factor = 1

K_2 = Terrain, Height Factor = 1

K_3 = Topography Factor = 1

$p = 0.6 \times V_z \times V_z$
= 0.6 x 50 x 50
= 1500 N/Sq.m
150 Kg/Sq.m



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5 **Design Loads:**

Slab 130 mm thk avg	=	3.3 KN/sq.m
Live load	=	2.0 KN/sq.m
Finishes	=	<u>1.7</u> KN/sq.m
		<u>7.0</u> KN/sq.m

Say 7.0 KN/sq.m/floor for slab and beam design

Applying live load reduction of = 0.2 ,

For column and foundation design, 5.6 Kn/sqm is taken.

This value is also taken for frame analysis

200 thick AAC blocks for exterior walls

$$0.2 \times 2.55 \times 8 = 4 \text{ KN/m/floor}$$

B.W. 100 thk for interior panels

$$0.100 \times 2.55 \times 8 = 2.04 \text{ KN/m/floor}$$

$$\text{Beams } 0.2 \times 0.35 \times 25 = 1.75 \text{ KN/m/floor}$$

$$\text{Columns } 0.3 \times 0.45 \times 3.00 \times 25 = 10.1 \text{ KN/floor}$$

These are taken for analysis with WL+LL+DL combination.



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6

Thickness	=	130 mm	Fck	= M 30	N/sq.mm
Cover	=	20 mm	Fy	=Fe 415	N/sq.mm
Eff.Depth	=	105 mm			

Panel (3.4 x 4.9) m

$$\text{Twoway slab} \quad L_y / L_x = 4.9 / 3.4 = 1.4$$

Support condition - 1 sides discontinuous

short span= α_x = 0.070 (- BM at continuous edge) Table:26
= 0.049 (+ BM at midspan) in IS-456-2000

Long span α_y = 0.055 (- BM at support)
0.042 +BM AT MIDSPAN

Designed for 0.070 co.efficient

$$M_u = 0.070 \times 7.0 \times 1.5 \times 3.4 \times 3.4 = 8.50 \text{ KNm/m}$$

$$\text{Mu} / \text{b.d.d} = \frac{8.50 \times 1000 \times 1000}{1000 \times 105 \times 105} = 0.77 \text{ N/sq.mm}$$

From Table-3 of SP-16, Pt 0.200 %

Minimum Pt = 0.12 % as per code, for Fe-415

$$At = 0.200 \times 1000 \times 105 \div 100 = \mathbf{210 \text{ sq.mm/m}}$$

Provide 10 dia @ 175 = **448.8** sq.mm/m Short span

Provide 8 dia @ 175 = **287.2 sq.mm/m** Long span

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8 Beam design:

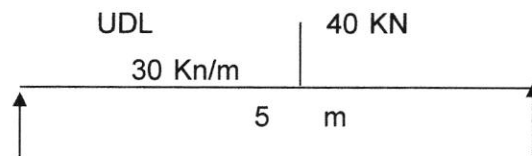
Beam-1

span = 5 m

Beam size 200 x 450

d = 410 mm

Fck = M 30



Bending moment for UDL 98.00 Kn.m

Max. B.M = 98.00 (top of support) This is taken for design

$$M_u = 98 \times 1.5 \times 10^{-6} = 147.0 \times 10^{-6} = \text{Nmm}$$

$$M_u/b.d.d = \frac{147.0 \times 10^{-6}}{200 \times 410 \times 410} = 4.37 \text{ N/sq.mm}$$

$$\text{From table-53 SP-16 for } d'/d = 40 / 450 = 0.00$$

$$P_t = 1.000 \% \quad P_c = 0.30 \%$$

$$A_t = 1.000 \times 200 \times 410 / 100 = 820 \text{ sq.mm (top)}$$

$$A_c = 0.300 \times 200 \times 410 / 100 = 246 \text{ sq.mm (bot)}$$

$$\text{provided } 3 \text{ No } 20 = 942 \text{ sq.mm} +$$

$$2 \text{ No } 12 = 226$$

$$1169 \text{ sq.mm at support top}$$

Shear Design:

$$\text{Shear force } p/2 = 95 \text{ KN}$$

$$\text{Factored shear force} = 1.5 \times 95 = 143 \text{ KN}$$

for 1.00 % steel Shear resisted by concrete section

$$\text{from table-61 } 0.65 \times 200 \times 410 / 1000 = 53 \text{ KN}$$

$$\text{Balance shear } (V_{us}) = 143 - 53 = 89 \text{ KN}$$

$$V_{us}/d = 89 / 41 = 2.18 \text{ KN/cm}$$

from table 62 of SP-16

Required 2 legged 8 stirrups @ 150 mm c/c

Provided 2 legged 8 stirrups @ 110 mm c/c

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9 **COLUMN DESIGN:** COL-A COL SIZE 300 X 600

Maximum load is with wind force $P = 160 \text{ t}$
 $P_u = 1.5 \times 160 \times 10 = 2400 \text{ KN}$
 Max.Bending Moment $= 3 \text{ t.m}$ $M_u = 1.5 \times 3.0 \times 10 = 45 \text{ Kn.m}$
 Cover $= 40 \text{ mm}$
 $d'/d = 0.07$
 $F_{ck} = 35 \text{ N/sq.mm}$

Provide bars equally all sides

Refer chart 44 of SP-16

$$\frac{P_u}{F_{ck} b d} = \frac{2400 \times 1000}{35 \times 300 \times 600} = 0.38$$

$$\frac{M_u}{F_{ck} b d^2} = \frac{45 \times 10^6}{35 \times 300 \times 600^2} = 0.01$$

$$\frac{p}{F_{ck}} = 0.04 \quad p\% = 0.05 \times 35 = 1.75 \%$$

Minimum %age of steel is 0.8 %

$$A_s = \frac{1.75 \times 300 \times 600}{100} = 3150 \text{ sq.mm}$$

$$\text{Provided } 6 \text{ Nos } 20 \text{ dia} = 1885 \text{ sq.mm}$$

$$+ 4 \text{ Nos } 25 \text{ dia} = 1964 \text{ sq.mm}$$

$$3848 \text{ SQ MM}$$

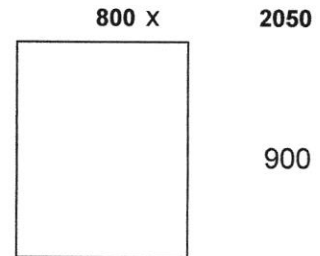
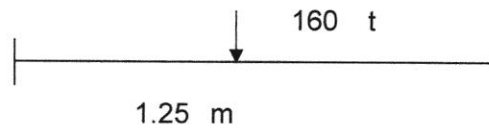
Ties 8 @ 125 & 175c/c


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10 Pile cap and pile

As per soil report, 15.5 m bored cast-in-situ pile can be adopted.

500 dia pile safe capacity = 100 t
 Col-A carries maximum load of = 160 t
 So No of piles provided = 2 Nos
 Pile cap size = 800 x 2050 X 800 MM
M-30 grade = 25 N/sq.mm



Section of pile cap

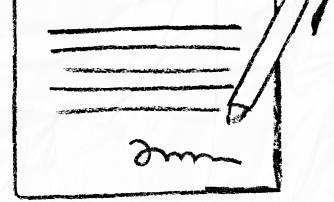
Pile to pile distance = 1.25 m

axial force = $160 \times 1.25 // 0.82$
 = 61 tonnes

At reqd = $60.98 / 2$ ' = 30 sqcm

Provided bttom bothways = **11 nos** 20
 = 34.6 sqcm


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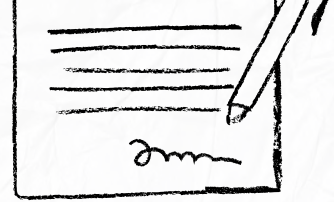
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