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STRUCTURAL DESIGN CALCULATION REPORT

THE PROPOSED CONSTRUCTION OF (GROUND + 2 FLOORS) RESIDENTIAL VILLAS PLOT NO: (9, 10, 11, 32, 33, 34, 35, 36 & ROW HOUSES 22 TO 27) AT SURVEY NO. 196/1A, T.S.NO.43, BLOCK NO.39 OF AMBATTUR VILLAGE(WARD-B) AMBATTUR TALUK, AND WITHIN THE LIMITS OF GREATER CHENNAI CORPORATION. ZONE:VII; DIVISION:085

FOR M/S. HI LIVING PROJECTS LLP

HI LIVING PRISTINE, AMBATTUR



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REV NO.	DATE	DESCRIPTION	STATUS	PREPARED BY	REVIEWED BY	APPRVD. BY
R0	13-02-24	STRUCTURAL DESIGN BASIS REPORT	OK	GOWTHAM	KALEEM	PERAM



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Structural Design Basis Report for the proposed construction of (Ground + 2 floors) Residential villas at Survey no.196/1A, 4.3 no.43, Block No.39 of Ambattur village (Ward-B) Ambattur Taluk and within the limits of Greater Chennai Corporation. Zone: VII, Division:085.

For M/S. Hilliving Projects LLP- Hilliving Pristine, Ambattur-Doc.No. APE-DBR-HRH-AN-CH-REV(0)

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at Survey no.196, Plot no.43, Block No.39 of Ambattur village (Ward-B) Ambattur Taluk and within the

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1. FORWARD

The intent of this document is to identify and record all the pertinent input requirements, analysis & design criteria for the structural design of the building. It is aimed at formulating the basis of the structural analysis, design & detailing work that the Structural Engineer is planning in delivering the structural scheme of the building. The scheme will be compatible with the architectural theme, satisfy the functional needs, at the same time conforming to the Indian Standards and other applicable building norms to achieve safe, stable, strong and yet optimally economical structures.

The parameters adopted in this report are going to be the basis of the structural design calculations. This report covers the minimum design requirement to establish the unified design basis that will form the overall design philosophy to be adopted in the structural design of the proposed building.

The design will aim to achieve

- ✚ Structural & functional integrity.
- ✚ Desirable Structural performance under characteristic service design loads.
- ✚ Desirable performance under the wind and earthquake forces as per the BIS provisions.
- ✚ Achieve Structural durability by selecting suitable materials and relevant requirements.

1.1 AGENCIES

Client : M/s. Hiliving Projects LLP.
Architect : M/s.Parag Kendrekar, Mumbai.
Structural Design Consultant : M/s. A P Engineers, Chennai.



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2. PROJECT DESCRIPTION

2.1 GENERAL

The proposed construction of (Ground + 2 floors) residential villas 12nos (plot no : 9, 10, 11, 32, 33, 34, 35, 36 & Row houses 22 to 27) at survey no. 196/1A, T.S.no.43, block no.39 of ambattur village(ward-b) ambattur taluk, and within the limits of greater Chennai corporation. Zone : VII ; division :085, Tamilnadu. It consist of Residential villas (Ground + 2 Storey structure). In which, all the floors are designed as residential units.

2.2 THE BUILDING

The proposed construction of a Residential villas consisting of Ground +2 floors. The Architectural & Structural configuration of the building is well defined and captured with lifestyle that fulfill the perfect living. The configuration of the building is given as below,

Table 2.1 : Architectural & Structural Configuration and Floor heights :

Residential Building	Ground +2 floors
Ground floor to first floor	3.05m
First floor to second floor	3.05m
Second floor to roof	3.2m

2.2.1 Architectural Configuration

Architecturally, it is configured fairly a linear **Rectangular** shaped buildings with the plan size of each Residential Building 6.77m width (Horizontal dimension) and 12.0m(Vertical dimension) above ground level. The Lift & Staircases are placed centre of the building which divides the building.

2.2.2 Structural Configuration

Structurally, the building is essentially a R.C.C. framed structure with columns and the building framing system is conceptualised with a conventional system of RCC Column connected by Floor Beams with slabs. The columns are connected to each other with a network of R.C.C beams and RC slabs with the slabs acting as in-plane rigid diaphragms for each of the floors. The Beams are assigned as frame elements in the 3D model. The structural system is designed with RC Columns which are connected by Beam-Slab. Based on the sub-soil condition and the load intensity of the vertical members, Isolated foundation is designed as Foundation system.



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3. LIST OF CODES

All design of structures shall comply with the requirements specified in the following.

- Relevant Indian Standard Codes of Practice
- National Building Code.
- Local Building regulations.
- Other relevant International codes where required.

All applicable latest editions of codes and standards will be referred and adopted in the analysis of structures and the design and detailing of structural members. All structural elements design shall be carried out primarily based on Indian Standards and Codes with latest revision, with amendments if any, as on date. In addition, other international codes and standards shall also be referred when required and relevant.

In the analysis, design and detailing of the building, the following Indian Standard codes of latest amendment will be used.

3.1 DESIGN LOADS (OTHER THAN EARTHQUAKE LOADS)

IS CODE	DESCRIPTION
IS 875 (Part 1):1987(Rev2018)	Code of practice for Design Loads (Other than Earthquake) for Buildings & Structures -Dead Loads (Unit Weight of Building Material and Stored Material)
IS 875 (Part 2):1987(Rev2018)	Code of practice for Design Loads (Other than Earthquake) for Buildings & Structures - Imposed Loads
IS 875 (Part 3):2015(Rev2020)	Code of practice for Design Loads (Other than Earthquake) for Buildings & Structures - Wind Loads
IS 875 (Part 4):1987(Rev2018)	Code of practice for Design Loads (Other than Earthquake) for Buildings & Structures - Snow Loads
IS 875 (Part 5):1987(Rev2018)	Code of practice for Design Loads (Other than Earthquake) for Buildings & Structures – Special Loads and Load Combinations

3.2 DESIGN FOR EARTHQUAKE RESISTANCE

IS CODE	DESCRIPTION
IS 1893 : 2016 Part-1	Criteria for Earthquake Resistance design of structures.
IS 4326:2013 (Rev-2018)	Earthquake Resistant Design and Construction of Buildings – Code of Practice
IS 13920:2016 (Rev-2021)	Ductile Detailing of Reinforced Concrete Structures subject to Seismic Forces - Code of Practice.
IS 16700 : 2017	Criteria for Structural Safety of Tall Concrete Buildings



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3.3 DESIGN OF STEEL STRUCTURAL ELEMENTS

IS CODE	DESCRIPTION
IS 800: 2007 (Rev-2021)	Code of Practice for General Construction in Steel
IS 2062: 2011 (Rev-2017)	Steel for General structural purposes
IS 3757 :1985 (Rev-2019)	Specification for High Strength Structural Bolts
IS 7215 : 1974 (Rev-2019)	Tolerances for Fabrication of Steel Structures
IS 7205 : 1974 (Rev-2021)	Safety Code for erection of Structural Steel work
IS 12843:1989 (Rev-2021)	Tolerances for Erection of Steel Structures

3.4 DESIGN OF CONCRETE ELEMENTS

IS CODE	DESCRIPTION
IS 456: 2000 (Rev-2016)	Plain and Reinforced Concrete - Code of practice
IS 2950-1981 (Reaff-2018)	Indian Standard Code of Practice for Design and Construction of Raft Foundation (Part – 1)
IS : 2911-Part-1(Sec-2)	Indian Standard Code of Practice for Design and Construction of Pile Foundation-Bored Cast In situ Piles
IS 2911-Part-4-2013	Design and Construction of Pile Foundation - Code of practice -Load Tests on Pile
IS 1904 : 2021	Indian Standard Code of practice for design & construction foundations in Soil : General Requirements
IS 2974 : 1982 (Rev-2018)	Code of practice for design & construction of machine foundation
SP 34	Handbook on Concrete Reinforcement & Detailing
SP 7(Part IV)-2016	National Building code of India
IS 1786-2008 (Rev-2018)	Specification for high strength deformed bars and wires for concrete Reinforcement.
IS 3370 (part I) (Rev-2019)	Code of practice for concrete structure for the storage of liquids-Part-1 : General Requirements
IS 3370(part II) (Rev-2019)	Code of practice for concrete structure for the storage of liquids-Part-2 : Reinforced Concrete structures

Table 3.5 Design of Concrete Elements - International codes

IS Code	Description
BS 8110 : 1997	Structural use of concrete. Code of Practice for design and construction
ACI 318	Building Code requirements for Structural Concrete



Structural Design Calculation Report of the proposed construction of (Ground + 2 floors) Residential villas at Survey no.196/1A, T.S no.43, Block No.39 of Ambattur village (Ward-B) Ambattur Taluk and within the

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4. DESIGN PARAMETERS

4.1 LOADS – SELF WEIGHT OF MATERIALS

Table-4.1.1 : Unit Weight of Materials

Material	Density
Reinforced Cement Concrete	25.00 kN/m ³
Plain Cement/Screed Concrete	24.00 kN/m ³
Cement Mortar	20.00 kN/m ³
Conventional Brick	20.00 kN/m ³
Structural Steel	78.50 kN/m ³
Cinder Concrete (Filling material)	08.00 kN/m ³
Dry soil	18.00 kN/m ³
Wet Soil	22.00 kN/m ³
Landscape soil	16.00 kN/m ³
Glass	28.00 kN/m ³
Water	10.00 kN/m ³

Table 4.1.2 –Conventional Brick wall loading

150mm thick wall with 20mm plaster on each side per meter height of wall	3.8 kN/m
200mm thick wall with 20mm plaster on each side per meter height of wall	4.80 kN/m

4.2 LOADS - IMPOSED LOADS ON SLAB

The following are the imposed loads in the structural slabs, in addition to the self-weight and the floor finishes. (Self-weight of slab will be as per the thickness mentioned in the respective drawings and the Floor finishes of 75mm/50 mm thickness have been considered on all floors for commercial/residential space respectively). Structural steel work shall conform to IS 226 and IS 2062 for Indian steel sections.

Table 4.2.1 Rooms

Load Component	Thickness (mm)	Load	Units
Floor Finishes+service		1.2	kN/m ²
Partition Loads		-	kN/m ²
Live Load		2.0	kN/m ²



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Table 4.2.2 Balcony

Load Component	Thickness (mm)	Load	Units
Floor Finishes		1.2	kN/m ²
Sunk & Filled		-	kN/m ²
Live Load		3.0	kN/m ²

Table 4.2.3 Toilets

Load Component	Thickness (mm)	Load	Units
Finishes		1.20	kN/m ²
Sunk & Filled	300 mm	6	kN/m ²
Live Load		2.00	kN/m ²

Table 4.2.4 Roof Terraces

Load Component	Thickness (mm)	Load	Units
Finishes		2.4	kN/m ²
Live Load		2.0	kN/m ²

Table 4.2.5 Staircases

Load Component	Thickness (mm)	Load	Units
Steps	150 (Riser)- 75mm Avg thickness	1.8	kN/m ²
Finishes	-	1.2	kN/m ²
Live Load	-	3.0	kN/m ²

Table 4.2.6 Lift Machine Rooms

Load Component	Thickness (mm)	Load	Units
Finishes		1.8	kN/m ²
Live Load		10.0	kN/m ²

5. MATERIALS



Structural Design Calculation Report of the proposed construction of (Ground + 2 floors) Residential villas at Survey no.196/1A, T-10, 43 Block No. 39 of Ambattur village (Ward-B) Ambattur Taluk and within the limits of Greater Chennai Corporation, Zone: VII, Division:085

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5.1 CONCRETE

Characteristic concrete strength (concrete grade) for the project shall be based on exposure condition and forces to be supported by specified structural elements. The minimum cement content and maximum water-cement ratio shall be as per IS code 456:2000. The minimum 28-day compressive strength of concrete for specified structural elements shall be as follows

Table 5.1.1 Grade of Concrete for Individual Structural elements

Sl.No.	Structural Elements	Min. Grade of Concrete
1.	Footing	M25
2.	RC Columns	M25
3.	RC Beams	M25
4.	RC Slabs	M25
5.	UG Tank	M25
6.	Other Structural	M25

Table 5.1.2 Concrete Properties

Young's Modulus of Elasticity of Concrete	$5000\sqrt{f_{ck}}$
Poisson Ratio's Ratio $\nu =$	0.2
Coefficient of Thermal expansion	5.5×10^{-6}

5.2 REINFORCEMENT

The characteristic tensile properties of steel used in design shall be as follows.

Reinforcement Characteristic yield strength f_{yk} (N/mm ²) for rest	500 N/mm ²
Young's Modulus E (kN/mm ²)	200 kN/mm ²
Poisson's ratio	0.3
Coefficient of Thermal expansion, α (oC-1)	12×10^{-6}

5.3 STRUCTURAL STEEL

All structural steel shall conform to Indian standard IS : 2062-2011.

Structural steel	Grade E=250	Grade E-350
Tensile strength	410 N/mm ²	490 N/mm ²
Yield stress $t < 20\text{mm}$	250 N/mm ²	350 N/mm ²
Yield stress $t = 20\text{mm to } 40\text{mm}$	240 N/mm ²	330 N/mm ²
Yield stress $t > 40\text{mm}$	230 N/mm ²	320 N/mm ²



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Young's Modulus (E)	200,000 N/mm ²	200,000 N/mm ²
Density	78.5 kN/m ³	78.5 kN/m ³

6. SUB-SOIL CONDITION

Soil investigation report no. VCE/23/GIR-170 dated Aug, 2023 referred for sub-soil condition & designing suitable foundation. As per report, the Soil Investigation carried out & the report prepared by Veltech Consulting Engineers. There are Four (4) Bore holes are investigated at different villas to ascertain the sub-soil condition at the proposed site through both field & lab tests. All boreholes are terminated at hard Clay-Shale layer encountered at a depth around 10m, where SPT (N) value encountered more than 100. As per the Bore hole data, the top layer is medium sand with silt exists with « N » value of 10. From 1.5m to 3m, it is a Reddish dark brown gravel sand with silt mixtures with « N » value is in the range of 27. From 3m to 6.5, it is Greyish brown medium plastic and medium swelling silty clay exists with « N » value ranging from 28 to 39. From 6.5 to 10m, it is a Greyish brown fine silty sand with clay binder exists with « N » value more than 100 where the bore hole is terminated. During the time of soil investigation (Aug 2022) the Ground Water Table (GWT) reported is 5m from existing ground level

Based on the sub-soil condition explored through the field and lab tests, consultant recommended to design Isolated Footing as Foundation system as the top soil is good in nature.

7. DESIGN CRITERIA

7.1 DESIGN LIFE

The design life of the building is assumed as 50 years. This requirement is applicable only for concrete works and not applicable to replaceable finishing materials, water proofing and thermal insulations, facades etc. Hence periodic maintenance/refurbishment is required for all replaceable materials.

7.2 TEMPERATURE EFFECTS

Temperature changes can cause additional stresses to the structure and has potential to increase in loads will be considered in the design of structural elements. Temperature effects will be considered for an ambient temperature variation from temperature during construction.

The maximum and minimum temperature for city of Chennai as recorded by Indian Metrological Department are as follows.

Maximum mean Temperature	45°C
Minimum mean Temperature	15°C



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Average ambient Temperature during construction	30°C
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The structural system will be designed for the effect of temperature variation. A temperature variation of $\pm 15^{\circ}\text{C}$ will be considered in the structural analysis for consideration of thermal effect on structural elements.

7.2.1 EXPANSION JOINTS

Above ground has been provided at suitable locations considering the size (length and width) of the building and the effect of temperature variation, with two (2) Expansion Joints (EJ) in order to split all the three structure independent.

7.3 WIND LOAD

Design wind load for the buildings shall be estimated as per IS 875 (Part 3): 2015. Design wind speed (V_z) is based on such as location, basic wind speed, terrain, cyclone condition and topography.

Location of the building	Chennai
Basic Wind speed - V_b m/sec	50 m/sec
Risk Coefficient (Probability factor) - k_1	1.0
Terrain factor (Terrain-3) - k_2 for the height of 11.85m	0.932
Topography factor - k_3	1.0
Importance factor (Cyclonic region) - k_4	1.0

Design wind speed (V_z) = $V_b \cdot k_1 \cdot k_2 \cdot k_3 \cdot k_4 = 50 \times 1 \times 0.932 \times 1.0 \times 1.0 = 46.6$ m/sec

Wind pressure (P_z) = $0.6 V_z^2 = 0.6 \times 46.6^2 = 1303$ N/m²

For design of building wind pressure 1.3 kN/m² is considered.

Design wind pressure (P_d) = $K_d \cdot K_a \cdot K_c \cdot P_z$

Where K_d - Wind Directionality factor = 1.0

K_a - Area averaging factor = 0.9

K_c - Combination factor = 0.9

Design Wind pressure = $1.3 \times 1 \times 0.9 \times 0.9 = 1.053$ kN/m²

7.4 DYNAMIC WIND ANALYSIS

As per IS 875 (Part 3) - 2015, clause 9.1 Dynamic wind analysis has to be carried out if the buildings meet the following criteria.

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a). Buildings and closed structures with a height to minimum lateral dimension ratio (H/W_{min}) is more than 5. (or)

b). Building and structures whose natural frequency in the first mode is less than 1.0 Hz.

(a). $(11.85m/6.77m)$ is $1.75 < 5$.

Hence Wind dynamic analysis not required.

(b) Fundamental period of the building = $T_a = 0.09H/(D)^{0.5} = (0.09 \times 11.85) / (6.77)^{0.5} = 0.410$

Natural frequency $1/T_a = 1/0.410 = 2.43 > 1.0$

Hence Wind dynamic analysis not required.

7.5 SEISMIC LATERAL FORCE

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$$A_w = \sum_{i=1}^{N_w} \left[A_{wi} \left\{ 0.2 + \left(\frac{L_{wi}}{h} \right) \right\}^2 \right]$$

where

h = height of building as defined in 7.6.2(a), in m;

A_{wi} = effective cross-sectional area of wall i in first storey of building, in m^2 ;

L_{wi} = length of structural wall i in first storey in the considered direction of lateral forces, in m;

d = base dimension of the building at the plinth level along the considered direction of earthquake shaking, in m; and

N_w = number of walls in the considered direction of earthquake shaking.

The value of L_{wi}/h to be used in this equation shall not exceed 0.9.

7.5.1 Design Parameters for estimation of seismic loading

Sl.No.	Description	Parameters as per IS-1893-2016
1.	Location - Chennai	Seismic zone III
2.	Seismic Zone Factor	0.16
3.	Response Reduction Factor : RC buildings with ordinary moment resisting frame (OMRF)	3.0
4.	Importance factor (Residential building)	1.2
5.	Type of Soil (hard)	Type 1 Rock
6.	Damping coefficient	5%



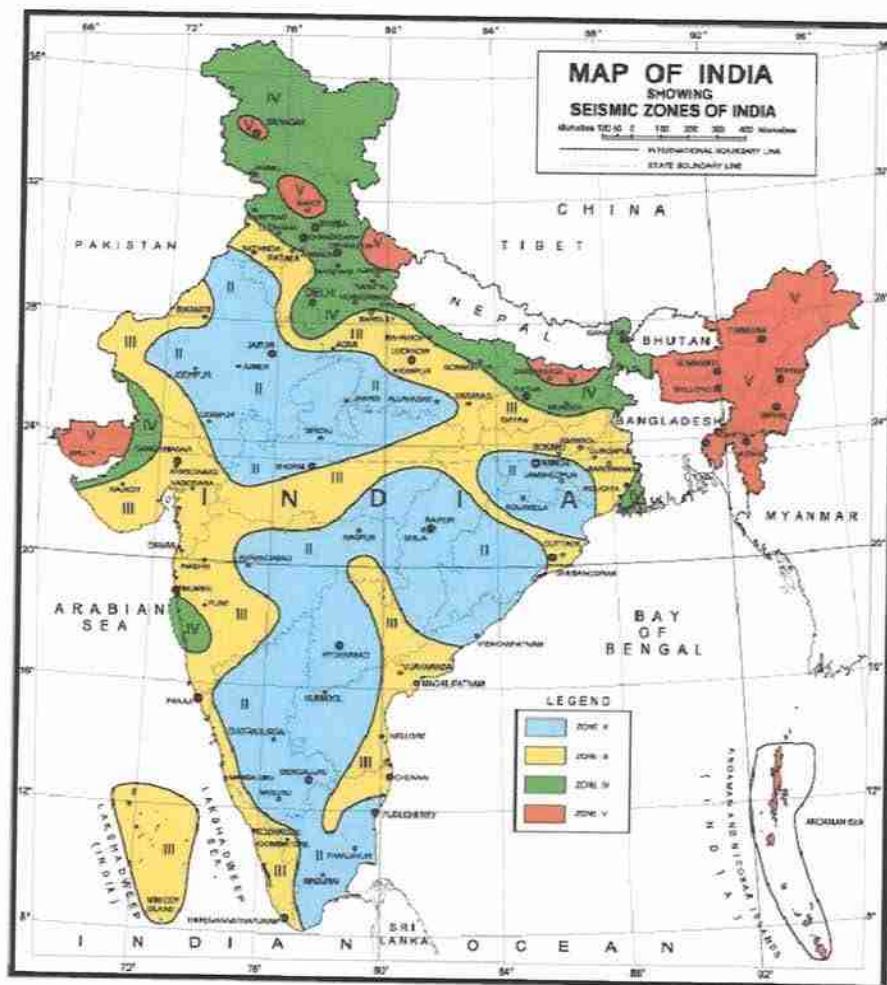
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7.5.2 BASE SHEAR

Height of the building (h) = 11.85m (measured from Ground floor)

d (along the length of the building) = 6.77m

b (along the breadth of the building) = 12.0m

a) T_a (along x) = $0.09h/d^{0.5}$

= 0.409 Sec

b) T_a (along y) = $0.09h/d^{0.5}$

= 0.307 Sec

From IS 1893: 2002, Clause 6.4.2, Design horizontal seismic coefficient

$$A_h = \frac{Z}{2} \times \frac{I}{R} \times \frac{S_a}{g}$$

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Where,

Z for Zone III = 0.16

I for Residential Building = 1.2 (Refer to IS 1893-2016, Table.8)

Response Reduction Factor for RC buildings with

Ordinary moment resisting frame (OMRF) = 3.0 as per (Table 9)

Sa/g (For Rocky) x direction & y direction = 0.25 (Refer to IS 1893-2016, page 13, Fig 1)

Design horizontal seismic coefficient and base shear

X- direction Ahx = 0.008

Vbx = 0.008W

Y- direction Ahy = 0.008

Vby = 0.008W

7.5.3 DYNAMIC ANALYSIS

Response spectrum method of Dynamic Analysis carried out with following parameters pertaining to dynamic analysis.

Time period	: Calculated with the assumption of Flat Slab with Structural wall System.
Number of modes Considered	: 12 Numbers (Min)
Modal Combination Method Used	: Complete Quadratic combination (CQC).
Spatial Combination Method used	: Square root of the sum of Square (SRSS).
Modal case type	: Ritz vector Method
Orthogonal directional seismic effect	: Not applicable

Accidental torsion considered as per IS: 1893-2016

Design Eccentricity

While performing structural analysis by the Seismic Coefficient Method or the Response Spectrum Method, the design eccentricity e_{di} to be used at floor i shall be taken as:

$$e_{di} = \begin{cases} 1.5e_{si} + 0.05b_i \\ e_{si} - 0.05b_i \end{cases}$$

8. Structural Analysis and Design



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8.1 ASSUMPTIONS CONSIDERED IN THE DESIGN

The assumptions for structural analysis and design are as below,

- Elastic analysis (plane remains plane).
- Structure shall be analysed for the most adverse load conditions and envelope of forces shall be designed for
- Cracked section properties of structural RC structural elements are used in the analysis as per IS 16700-2017, clause 7.2, Table 6.

8.2 CONCRETE COVER

Concrete cover to the reinforcement in structural elements is based on durability and fire rating. The fire rating of 2hrs is considered for structural elements for the commercial development.

Sl. No.	Structural Elements	Cover in mm
1.	Footing	50
2.	Columns	40
3.	Beams	35
4.	Slabs	25
5.	UG Tank Base Slab/Wall	40

8.3 METHOD FOR ANALYSIS

Computer-aided analysis using finite element method (FEM) shall be carried out for the design of structural elements. For 3D or 2D structural frame analysis latest version of software ETABS, SAFE shall be used, Prokon, RCDC and verified in-house spread sheets will be used for the design of reinforced concrete (RC) and steel elements in consideration of flexure, shear and axial forces. RAPT will be used for the design of pre-stressed concrete elements.

8.3.1 ASSUMPTIONS FOR 3D MODEL ANALYSIS USING ETABS

Columns : Frame Element
 Beam Element : Frame Element
 Slab : Membrane

The following are general parameters adopted in the analysis and design,
 Modulus of Elasticity of Concrete -5000√fck

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Wall element considered as Shell element

The following parameters and methods are adopted in creating ETABS 3D model for the buildings and design of structural elements.

- The 3D ETABS model generated based on the preliminary floor plans (shuttering drawings) prepared using preliminary sizes.
- Sectional properties are assigned to all the structural elements as per the preliminary floor plans (shuttering drawings).
- External & Internal in fill wall loads are assigned as uniformly distributed line load on beams and as area load if the wall is intend to distribute on slab directly. These loads are assigned under SIDL type load case.
- Slabs are modelled as shell element if floor system is Flat slab and the slab has been modelled as membrane in Beam-slab as floor system and slabs are assigned with horizontal diaphragms.
- Live Load reduction is considered in the design of Foundation and Column as per the Code : IS 875-1987 (Part-II), clause.3.2.1.
- Area design load i.e. Live load/SIDL or any other load is applied at floor relevant to its usage.
- All lateral loads & parameters are assigned as per Codes IS 1893-2016 & IS 875-2015.
- Temperature loads are applied on to the terraces and to areas directly exposed to sun.
- Beams under Torsion is required to maintain Equilibrium will be designed for Torsion in accordance with IS 456-2000, clause 41.1 to 41.4 and for all other beams torsional stiffness is neglected in the analysis by applying stiffness modifier as 0.01 to torsional constant "J". Hence no specific design for Torsion is required for these beams.
- Chennai is zone-III. Hence ductile design and detailing is followed as per the IS-13920-2016 code recommendation.
- Concrete cover to reinforcement complied accordance with IS 456-2000, Tabl.16 & 16A.
- UG Sump and STP is designed for the crack width of 0.2mm whereas the Building structural elements are designed for the crack width of 0.3mm.
- Secondary beams are not designed for ductile detailing.
- Long term deflections are checked by manual calculation using excel spread sheet wherever necessary. Also crack width calculation will be done using spread sheet wherever necessary.

Area Load & Application : Applied as uniformly distributed area load with intensity proportionate to its usage as mentioned in the Loading items in the DBR.

Line (Wall) Load & Application : non load bearing or infill wall loads are calculated appropriately and Applied as uniformly distributed line load.

Lateral Load Resisting Systems : The proposed structure conceptualised to be "Buildings with Ductile RC Structural walls and Special Moment Resisting Frame (OMRF)". In this case the lateral

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load will be transferred primarily by RC walls and Columns for proportion of its stiffness in the direction considered. Response factor used "4" as per IS-1893-2016.

9. LOAD COMBINATIONS

The various loads shall be combined in accordance with the stipulations in IS 875 (Part 5) 1987 and IS 456: 2000. The structure will be checked for serviceability Limit State and Ultimate Limit State (Limit State of Collapse) conditions. The load factors to be used for the load combinations are shown in the Table below. The structural elements shall be designed and checked for the critical forces from the combinations.

Load Combination	DL	IL	TL	Spec X	Spec Y	WX	WY
Serviceability Limit State- combinations							
LC1 (D.L + T.L.)	1.0	-	1.0	-	-	-	-
LC2 (D.L + I.L + T.L.)	1.0	1.0	1.0	-	-	-	-
LC3 (D.L + T.L + Spec X)	1.0	-	1.0	1.0	-	-	-
LC4 (D.L + T.L + Spec Y)	1.0	-	1.0	-	1.0	-	-
LC6 (D.L + T.L + W X)	1.0	-	1.0	-	-	1.0	-
LC7 (D.L + T.L - W X)	1.0	-	1.0	-	-	-1.0	-
LC8 (D.L + T.L + W Y)	1.0	-	1.0	-	-	-	1.0
LC9 (D.L + T.L - W Y)	1.0	-	1.0	-	-	-	-1.0
LC10 (D.L + I.L + T.L + Spec X)	1.0	0.8	1.0	0.8	-	-	-
LC11 (D.L + I.L + T.L + Spec Y)	1.0	0.8	1.0	-	0.8	-	-
LC12 (D.L + I.L + T.L + W X)	1.0	0.8	1.0	-	-	0.8	-
LC13 (D.L + I.L + T.L - W X)	1.0	0.8	1.0	-	-	-0.8	-
LC14 (D.L + I.L + T.L + W Y)	1.0	0.8	1.0	-	-	-	0.8
LC15 (D.L + I.L + T.L - W Y)	1.0	0.8	1.0	-	-	-	-0.8
Ultimate Limit State – Combinations							
Comb1 (D.L + T.L)	1.5	-	1.0	-	-	-	-
Comb2 (D.L + I.L + T.L)	1.5	1.5	1.0	-	-	-	-
Comb3 (D.L + I.L + T.L + Spec X/Spec XC / Spec XW)	1.2	1.2	1.0	1.2	-	-	-
Comb4 (D.L + I.L + T.L + Spec Y/Spec YC / Spec YW)	1.2	1.2	1.0	-	1.2	-	-
Comb5 (D.L + I.L + T.L + W X)	1.2	1.2	1.0	-	-	1.2	-
Comb6 (D.L + I.L + T.L - W X)	1.2	1.2	1.0	-	-	-1.2	-
Comb7 (D.L + I.L + T.L + W Y)	1.2	1.2	1.0	-	-	-	1.2
Comb9 (D.L + I.L + T.L - W Y)	1.2	1.2	1.0	-	-	-	-1.2
Comb10 (D.L + T.L + Spec X/Spec XC / Spec XW)	1.5	-	1.0	1.5	-	-	-
Comb12 (D.L + T.L + Spec Y/Spec YC / Spec YW)	1.5	-	1.0	-	1.5	-	-
Comb13 (D.L + T.L + Spec X/Spec XC / Spec XW)	0.9	-	1.0	1.5	-	-	-
Comb14 (D.L + T.L + Spec Y/Spec YC / Spec YW)	0.9	-	1.0	-	1.5	-	-
Comb15 (D.L + T.L - Spec Y)	1.5	-	1.0	-	-1.5	-	-
Comb16 (D.L + T.L + W X)	1.5	-	1.0	-	-	1.5	-
Comb17 (D.L + T.L - W X)	1.5	-	1.0	-	-	-1.5	-
Comb18 (D.L + T.L + W Y)	1.5	-	1.0	-	-	-	1.5



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Comb19 (D.L + T.L – W Y)	1.5	-	1.0	-	-	-	-1.5
Comb20 (D.L + T.L + W X)	0.9	-	1.0	-	-	1.5	-
Comb21 (D.L + T.L – W X)	0.9	-	1.0	-	-	-1.5	-
Comb22 (D.L + T.L + W Y)	0.9	-	1.0	-	-	-	1.5
Comb23 (D.L + T.L – W Y)	0.9	-	1.0	-	-	-	-1.5

DL: Dead Load IL : Imposed Load TL – Temperature Load

Spec X: Earthquake Load in X-dir. WX: Wind Load in X-dir.

Spec Y: Earthquake Load in Y-dir. WY: Wind Load in Y-dir.

10 ACCEPTANCE CRITERIA

Under Serviceability Limit State the limiting structural deflection shall be as follows.

Total deflection under all imposed loads including temperature, creep and shrinkage shall not exceed $L/250$.

In the above limiting value, the deflection contribution due to temperature, creep and shrinkage shall not be more than $L/350$.

Lateral deflection (sway) due to wind shall not exceed $H/500$ ($0.002 H$) where H is the height of the building above ground.

10.1 CRACK WIDTH LIMITS

The Limiting values of crack width as per clause 35.3 of IS : 456-2000 is followed in the design

- In general, For all Buildings structural elements includes slabs, beams etc, the crack width is limited to 0.3mm.
- For water retaining structures and Basement-3 slab/wall elements the crack width is limited to 0.2mm.

11 Design of Structural Elements and Foundation

11.1 FOUNDATION DESIGN

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The proposed Residential Building comprises Ground+2 Storey structure. The vertical elements like Columns are moderately loaded. The structure is provided with columns are adequately spreaded across in order to satisfy the structural requirements for gravity loads.

As per the soil test report recommendation, designed Isolated Footings as Foundation system. Safe Bearing Capacity used in the design of Footings are 25 t/m^2 as per soil report. The Footings are rest over the medium dense with « N » value is more than 27 at 2m depth from measured from EGL.

11.2 COLUMN

RC Structural Walls & Columns which are supporting both vertical loads and lateral forces are designed for the most adverse load conditions and envelope of the forces which will be the maximum for that structural element from the load combinations considered in the 3D analysis. ETABS/RCDC a design software will be used to design the element for the maximum forces obtained from the load combinations. As the building is located in Zone III, ductile details shall be followed.

11.3 BEAMS

Each Beam will be designed for maximum forces from the 3D analysis and RCDC or RAPT Pre-stress concrete design software shall be used to design of RC members. As the building is located in Zone III, ductile details shall be followed.

11.4 SLABS

Slab will be designed using SAFE software. Other regular floor slabs will be designed using in-house spread sheet and unique/special shaped slabs will be designed using SAFE.

12 EXTERNAL WORKS – ROADS, DRAINS & COMPOUND WALL

12.1 Road works

Road pavement can be flexible pavement or rigid pavement. Road pavement consists of subgrade, sub-base, base course, asphalt base course and asphalt wearing course. Subgrade is the soil layer at road formation level, sub-base and base course compacted approved graded aggregate or other suitable material and over which asphalt will be laid. Asphalt layer consists of asphalt base course and asphalt wearing course.



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For rigid pavement approved concrete mix laid base course in place of asphalt. Specification for works will be provided separately.

12.2 Drains

Depending on the layout of rain water drainage system, reinforcement details for drains and culverts as required will be provided.

12.3 Compound wall

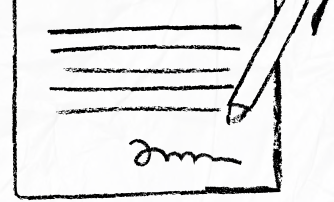
As per the architectural intend the compound wall will be designed. If it is required it will be designed to retain soil for certain height. It will also be designed for the lateral pressure. Foundation could be designed Isolated footing taking the soil/site condition in to account.


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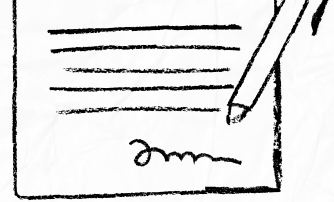
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Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator



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