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GEOTECHNICAL INVESTIGATION REPORT
FOR
PROPOSED CONSTRUCTION OF APARTMENT BUILDING

JOB NO: YPI/SIR/2020-21/262

SITE NAME: PROPOSED CONSTRUCTION OF APARTMENT BUILDING

SITE ADDRESS: PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT.

LAT-LONG: 11.046207, 76.930172

SITE INVESTIGATION DATE: 29.03.2021

REPORT SUBMITING DATE: 31.03.2021

CLIENT

M/S. JMJ HOUSING,
COIMBATORE

YUBE
Prime Infra.

" Expert in Geo-technical Services "

No 48, 1st Floor, Palaniappa Nagar, Suramangalam Post, Salem, Tamilnadu – 636 005
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SOIL INVESTIGATION REPORT

CERTIFIED ISO 9001:2015 COMPANY

Job No: YPI/SIR/2020-21/262
Date: 28.03.2021

To

M/s.JMJ HOUSING,
COIMBATORE.

Name of Work : GEOTECHNICAL INVESTIGATION WORK FOR PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT.

Sub : Submission of Soil Investigation Report - Reg

Dear Sir,

We are pleased to hereby transmit Two (02) original Hard Copies and Soft Copies sent to your concern address, our final Geotechnical Investigation Report for the above mentioned project. If you have any questions, please contact this office.

It has been a pleasure being of service to you on this project. Assuring you of our continued co-operation, we remain.

YUBE PRIME INFRA * SALEM - 636 005 *

For Yube Prime Infra.,

(Authorized Signatory)

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1 INTRODUCTION

Sub surface investigation and laboratory tests for PROPOSED CONSTRUCTION OF APARTMENT BUILDING at S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT.

The objective of the exploration work was to determine the probable sub surface conditions such as stratification, denseness or hardness of the strata, position of ground water table etc. and to evaluate probable range of safe bearing capacity for the structure. To fulfill the objective, the work carried out is comprises of:

- Drilling three boreholes up to the maximum depth of 10.00m below existing ground level in order to know the sub surface stratification, conducting necessary field tests and to collect disturbs and undisturbed soil samples for laboratory testing.

- Testing soil samples in the laboratory to determine its physical and engineering properties of the soil samples, and

- Analyzing all field and laboratory data to evaluate safe bearing capacity of the soil for given foundation sizes and necessary recommendations for foundation design and construction.

2 PROCEDURE OF INVESTIGATION

The actual site investigation work was started on 29.03.2021 and was completed on 30.03.2021.

2.1 Drilling

Three boreholes of 150mm diameter is drill up to the maximum depth of 10.00m. Where caving of the borehole occurred, casing was used to keep the borehole stable. The work was in general accordance with IS: 1892 – 1979. Client's representatives choose and selected the location of borehole.

2.1.1 Disturbed Samples: Disturbed representative samples were collected, logged, labelled and placed in polythene bags.

2.1.2 Undisturbed Samples: Undisturbed soil samples are collected in 100 mm diameter thin walled sampler (Shelby tube) from the borehole. The sampler used for the sampling had smooth surface and appropriate area ratio and cutting edge angle thereby minimizing disturbance of soil during sampling. Samples are logged and labelled properly and transfer to the laboratory for further testing.

2.1.3 Water Table: Water table was not met at during the sub soil exploration work carried out in the month of **MAR-2021**

2.1.4 Seismic Zone: The proposed project is situated at **KAVUNDAMPALAYAM VILLAGE, COIMBATORE DISTRICT.** It falls under the Seismic **Zone - III**, according to the Indian Standard Seismic Zoning Map. Zone -III: This is said to **Moderated Seismic Zone.**

2.1.5 Method of Sampling: Sampler is coupled together with a sampler head to form a sampling assembly. The sampler head provide a non-flexible connection between the sampling tube and the drill rods. Vent holes are provided in the sampler head to allow escape of water from the top of sampler tube during penetration. The sampling tubes are made free from dust and rust. Coating of oil is applied on both sides to obtain the undisturbed samples in best possible manner.

The sampler is then lowered inside the bore hole on a string of rods and driven to a predetermined level. On completion of driving the sampler is first rotated within the borehole to shear the soil sample at bottom and then pulled out. Upon removal of the sampling tubes, the length of sample in the tube is recorded. The disturbed material in the upper end of the tube, if any, is completely removed before sealing.

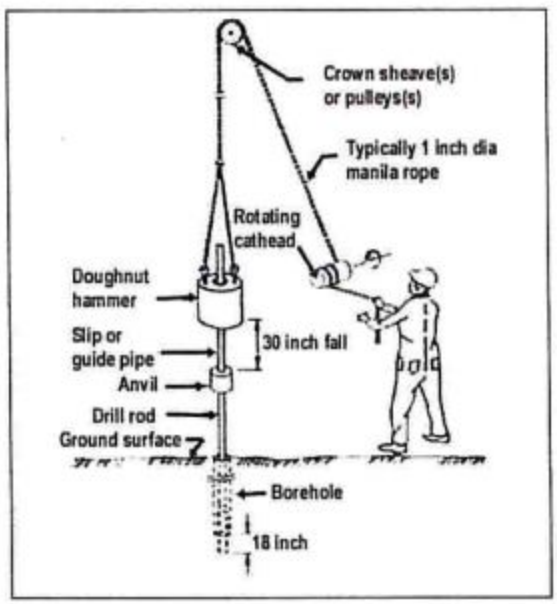
The soil at the lower end of the tube is trimmed to a distance of about 10 to 20 mm. After cleaning and inserting an impervious disc at each end, both ends are sealed. The empty space in the sampler, if any, is filled with the moist soil, and the ends covered with tight wrapper. The identification mark is then made on each sample.

2.2 Standard Penetration Test (SPT)

A standard penetration tests were conducted at 1.0/1.5 m intervals or at change of stratum as per Indian standard IS 2133-1981 and split spoon sampler used confirmed to the Indian standard, IS 9640-1980.

The standard penetration test is conducted in a borehole using a Standard split spoon sampler. When the borehole has been drilled to the desired depth, the drilling tools are removed and the sampler is lowered to the bottom of the hole. The sampler is driven into the soil by a drop hammer of 63.5 kg mass falling through a height of 75 cm at the rate of 30 blows per minute as per IS 2133-1963. The number of hammer blows required to drive 15 cm of the sample is counted. The sample is further driven by 15 cm and the number of blows recorded. Likewise, the sample is once again further driven by 15 cm and the number of blows recorded. The number of blows recorded for the first 15 cm is disregarded. The number of blows recorded for the last two 15 cm intervals are added to give the standard penetration number (N). If the number of blows for 30 cm drive exceeds 50, it is taken as refusal and the test is discontinued.

Sufficient disturbed soil samples were collected from the split spoon sampler while performing the standard penetration tests in each hole.



2.3 Laboratory Tests

The operations to be performed in the laboratory depend upon the type of the nature of data required for the problem at hand. In case of cohesionless material, like sand, the laboratory tests are usually minimum and the design parameters are worked out from field test data such as form SPT- N value, Core resistance and plate load test data.

In the case of cohesive soils, the programme of laboratory testing can vary from carrying out simple tests such as unconfined tests to comprehensive study of soil behaviour using triaxial shear tests and consolidation tests. Following laboratory tests are conducted.

COHESIVE SOIL	COHESIVE LESS SOIL
Specific gravity	Specific gravity
Natural moisture content	Natural moisture content
Sieve Analysis	Sieve analysis
Unconfined compression test	Direct shear test
Wet / Dry Density	Wet / Dry Density
Atterberg's limits	
Consolidation test	
Triaxial test	

3 RELEVANT IS – CODES

All the test are carried out as per IS codes. Some of the prominent test and there relevant IS Codes are given in the table.

Sl. No	Type of Testing	As per IS Code
01	Excavation of bore holes (Field Test)	IS 1892-1979
02	Standard Penetration Test (Field Test)	IS 2133-1981
03	Atterberg' s Limit (Laboratory Test)	IS 2720 Part 5-1970
04	Grain Size Analysis (Laboratory Test)	IS 2720 Part 4-1975
05	Specific Gravity Test (Laboratory Test)	IS 2720 Part 3-sec2-1981
06	Density Test (Laboratory Test)	IS 2720 Part 7 -1983
07	Direct Shear Test (Laboratory Test)	IS 2720 Part 13-1972
08	Tri-axial Compression Test (Laboratory Test)	IS 2720 Part 10-1973 IS 2720 Part 11-1971 IS 2720 Part 12-1981
09	Soil Classification (Laboratory Test)	IS 1498-1970
10	Soil Bearing Capacity as per settlement criteria	IS 8009 Part 1-1976
11	Soil Bearing Capacity as per shear criteria	IS6403-1981

4 FOUNDATION ANALYSIS

4.1 Soil Profile

The profile of the area as observed in the bore hole is given in the chart. The Borehole is terminated at maximum depth 10.00m.

Borehole No: 01

Depth of Soil (m)	Soil Description	IS Classification
0 – 3.60	Silty Sand	SM - Silty Sand
3.60 – 7.40	Sandy Clay	SC - Sandy Clay
7.40 – 10.00	Silty Sand	SM - Silty Sand

Borehole No: 02

Depth of Soil (m)	Soil Description	IS Classification
0 – 0.70	Filled Up Soil	-
0.70 – 10.00	Silty Sand	SM - Silty Sand

Borehole No: 03

Depth of Soil (m)	Soil Description	IS Classification
0 – 0.90	Filled Up Soil	-
0.90 – 10.00	Silty Sand	SM - Silty Sand

4.2 Safe Bearing Capacity

Maximum intensity of loading that the foundation will safely carry without the risk of shear failure of soil irrespective of any settlement that may occur.

Safe Bearing Capacity (SBC) = $q = 0.314\ N^2\ B.Rw2 + 0.943\ (100 + N^2)\ Df.\ Rw1$

Where,

N	:	Standard Penetration Value
B	:	Width of Footing (or least lateral dimensions)
Df	:	Depth of Foundation
Rw1 & Rw2	:	Water Reduction Factor

4.3 Allowable Bearing Pressure

The net intensity of loading which the foundation will carry without undergoing settlement in excess of the permissible value for the structure under consideration but not exceeding net safe bearing capacity. The safe bearing pressure, qs corresponding to a settlement of 25mm is given by,

$$q = 1.36\ (N - 3)\ (B+0.3/2B)^2\ Rw2\ .\ Rd\ S$$

Where,

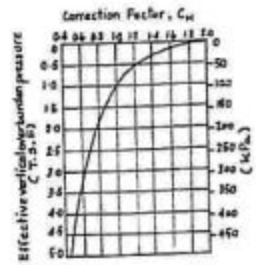
B	:	Maximum Size of the Footing
R	:	Water table correction factor (For the worst case, Taken as 0.5)
S	:	Allowable Settlement, S=25mm for Sandy soil, 40mm for Clay soil

SBC CALCULATION

BOREHOLE 01

Allowable bearing capacity calculation

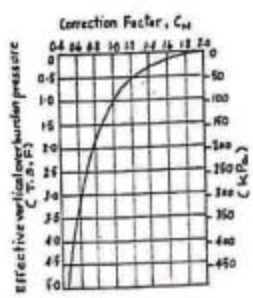
Width of the footing, B	=	2 m
Depth of foundation, Df	=	2 m
Unit weight of soil	=	17 kN/m^3
Submergible unt wt	=	7 kN/m^3
Avg. observed N value	=	25
Eff. overburden pressure at foundation level	=	14 kN/m^3
Overburden correction factor	=	1.6
N value after overburden corretion, N'	=	40
Depth of GWT	=	10 m
Dialtancy corretion, N"	=	40
Factor of Safety	=	3
Rw1	=	0.5
Rw2	=	0.5
Ultimate Bearing capacity based on shear	=	$0.314xN''^2xBxRw1+0.943x(100+N''^2)xDfxRw2$
	=	2105.5 KN/m^2
Safe bearing capacity based on shear	=	701.833 KN/m^2
Safe bearing capacity based on settlement	=	$1.36x(N''-3)x((B+0.3)/2B)^2xSxRdxRw)$
Settlement for Sand	=	25 mm
Depth factor, Rd	=	1+Df/B <2
Rd	=	1.1 Say
Water table factor, Rw	=	0.5+0.5(Dw/B) 0.5<Rw<1
Rw	=	0.5 Since GWT is below 10m from EGL
Qs	=	228.759 kN/m^2 For 25mm controlled Settlement
Allowable Bearing Pressure at Df depth is	=	**200** kN/m^2



BOREHOLE 02&03

Allowable bearing capacity calculation

Width of the footing, B	=	2 m
Depth of foundation, Df	=	2 m
Unit weight of soil	=	18 kN/m^3
Submergible unt wt	=	8 kN/m^3
Avg. observed N value	=	50
Eff. overburden pressure at foundation level	=	16 kN/m^3
Overburden correction factor	=	1.4
N value after overburden corretion, N'	=	70
Depth of GWT	=	10 m
Dialtancy corretion, N"	=	70
Factor of Safety	=	3
Rw1	=	0.5
Rw2	=	0.5
Ultimate Bearing capacity based on shear	=	$0.314xN''^2xBxRw1+0.943x(100+N''^2)xDfxRw2$
	=	6253.6 KN/m^2
Safe bearing capacity based on shear	=	2084.53 KN/m^2
Safe bearing capacity based on settlement	=	$1.36x(N''-3)x((B+0.3)/2B)^2xSxRdxRw)$
Settlement for Sand	=	25 mm
Depth factor, Rd	=	1+Df/B <2
Rd	=	1.1 Say
Water table factor, Rw	=	0.5+0.5(Dw/B) 0.5<Rw<1
Rw	=	0.5 Since GWT is below 10m from EGL
Qs	=	414.24 kN/m^2 For 25mm controlled Settlement
Allowable Bearing Pressure at Df depth is	=	**300** kN/m^2



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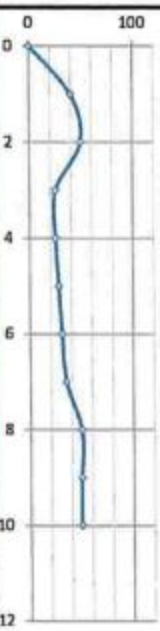
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BOREHOLE LOG

Client	: JMJ HOUSING, COIMBATORE.,	Ground Level	: 0.00
Project	: PROPOSED CONSRUCTION OF APARTMENT BUILDING	Water Table	: Not Met
Site	: S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT	Type Of Boring	: Rotary
B.H. No	: 01	Dia of Boring	: 150mm
Location	: MENTIONED ON SITEPLAN	Started on	: 29.03.2021
Final Depth	: 10.00m	Ended on	: 29.03.2021

Depth (m)	R.L	GWL	SYMBOL	DESCRIPTION	Depth of testing	SPT COUNT 0 - 15 cm	15 - 30 cm	30 - 45 cm	N - Value	Relative Density / Consistency
0.0	0.000									
1.0	-1.000			SILTY SAND	1.00	13	16	24	40	Dense
2.0	-2.000				2.00	50/5	-	-	>50	VDense
3.0	-3.000				3.00	9	11	14	25	Medium
4.0	-4.000				4.00	9	9	16	25	VStiff
5.0	-5.000			SANDY CLAY	5.00	10	11	17	28	VStiff
6.0	-6.000				6.00	10	12	19	31	VStiff
7.0	-7.000				7.00	12	15	20	35	Hard
8.0	-8.000				8.00	50/10	-	-	>50	VDense
9.0	-9.000			SILTY SAND	9.00	R				
10.0	-10.000				10.00	R				

Graphical Representation of Penetration Resistance



Bore hole terminated at 10.00m from the EGL

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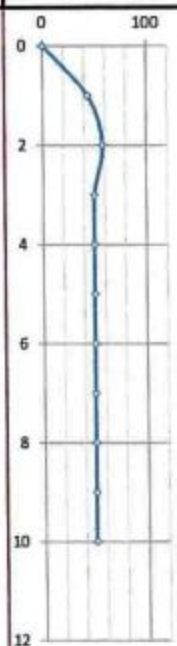
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BOREHOLE LOG

Client	: JMJ HOUSING, COIMBATORE.,	Ground Level	: 0.00
Project	: PROPOSED CONSRUCTION OF APARTMENT BUILDING	Water Table	: Not Met
Site	: S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT	Type Of Boring	: Rotary
B.H. No	: 02	Dia of Boring	: 150mm
Location	: MENTIONED ON SITEPLAN	Started on	: 29.03.2021
Final Depth	: 10.00m	Ended on	: 29.03.2021

Depth (m)	R.L	GWL	SYMBOL	DESCRIPTION	Depth of testing	SPT COUNT 0 - 15 cm	15 - 30 cm	30 - 45 cm	N - Value	Relative Density / Consistency
0.0	0.000			FILLED UP SOIL						
0.5										
1.0	-1.000				1.00	15	20	24	44	Medium
1.5										
2.0	-2.000				2.00	18	26	32	58	Dense
2.5										
3.0	-3.000				3.00	50/9	-	-	>50	do
3.5										
4.0	-4.000				4.00	50/10	-	-	>50	
4.5										
5.0	-5.000				5.00	R				
5.5										
6.0	-6.000				6.00	R				
6.5										
7.0	-7.000			SILTY SAND	7.00	R				
7.5										
8.0	-8.000				8.00	R				
8.5										
9.0	-9.000				9.00	R				
9.5										
10.0	10.000				10.00	R				

Graphical Representation of Penetration Resistance



Bore hole terminated at 10.00m from the EGL

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BOREHOLE LOG

Client	: JMJ HOUSING, COIMBATORE.,	Ground Level	: 0.00
Project	: PROPOSED CONSRUCTION OF APARTMENT BUILDING	Water Table	: Not Met
Site	: S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT	Type Of Boring	: Rotary
B.H. No	: 03	Dia of Boring	: 150mm
Location	: MENTIONED ON SITEPLAN	Started on	: 30.03.2021
Final Depth	: 10.00m	Ended on	: 30.03.2021

Depth (m)	R.L	GWL	SYMBOL	DESCRIPTION	Depth of testing	SPT COUNT 0 - 15 cm	15 - 30 cm	30 - 45 cm	N - Value	Relative Density / Consistency	Graphical Representation of Penetration Resistance
0.0	0.000										
0.5				SILTY SAND	1.00	16	25	32	57	V Dense	
1.0	-1.000										
1.5	-2.000				2.00	50/10	-	-	>50	do	
2.0											
2.5	-3.000				3.00	50/10	-	-	>50	do	
3.0											
3.5	-4.000				4.00	R					
4.0											
4.5	-5.000				5.00	R					
5.0											
5.5	-6.000			SILTY SAND	6.00	R					
6.0											
6.5	-7.000				7.00	R					
7.0											
7.5	-8.000				8.00	R					
8.0											
8.5	-9.000				9.00	R					
9.0											
9.5											
10.0	-10.000				10.00	R					



Bore hole terminated at 10.00m from the EGL

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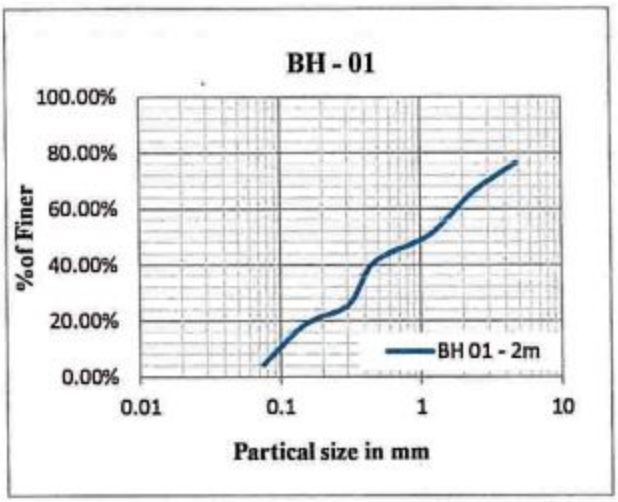
ANNEXURE II
LABORATORY INVESTIGATION REPORT

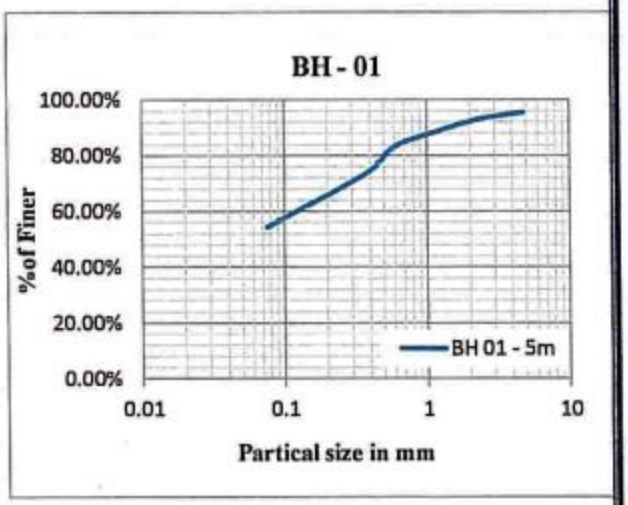
BH No	Depth (m) from EGL	Method of Sample	Moisture Content %	Grain Size Analysis			Consistency limits			Shear Test		Specific Gravity	Density g/cc	IS Classification	Observed N Value	Corrected N Value	Safe Bearing Capacity T/m2	Core Recovery %	Rock Quality Designation %	Ultimate crushing strength (mpa)
				Gravel %	Sand %	Silt & Clay %	Liquid Limit %	Plastic Limit %	Plasticity Index %	Cohesion value C kg/cm2	Angle of Internal Friction									
01	1.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	40	42.30	29.74	-	-	-
	2.00	SPT	9.97	23.72	72.0	4.3	NP	NP	NP	-	38	2.63	1.8	SM	>50	50.00	38.13	-	-	-
	3.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	25	26.78	19.67	-	-	-
	4.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	25	25.23	19.22	-	-	-
	5.00	SPT	14.9	4.50	41.0	54.5	42	25	17	140	-	2.65	1.7	SC	28	26.05	20.53	-	-	-
	6.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SC	31	26.97	21.91	-	-	-
	7.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SC	35	28.62	23.84	-	-	-
	8.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	48.33	-	-	-
	9.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	50.03	-	-	-
	10.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	51.73	-	-	-
02	1.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	44	45.78	32.31	-	-	-
	2.00	SPT	20.8	24.85	69.2	5.9	NP	NP	NP	-	38	2.59	1.8	SM	58	55.78	40.47	-	-	-
	3.00	SPT	12.2	4.23	82.0	13.8	NP	NP	NP	-	38	2.62	1.8	SM	>50	50.00	39.83	-	-	-
	4.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	41.53	-	-	-
	5.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	43.23	-	-	-
	6.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	44.93	-	-	-
	7.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	46.63	-	-	-
	8.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	48.33	-	-	-
	9.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	50.03	-	-	-
	10.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	51.73	-	-	-

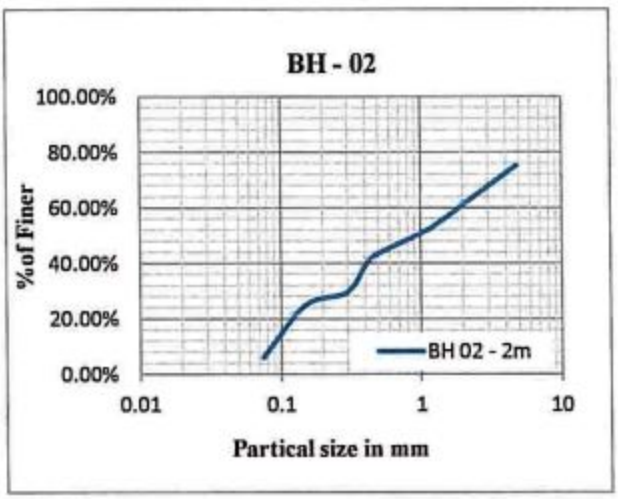
BH No	Depth (m) from EGL	Method of Sample	Moisture Content %	Grain Size Analysis			Consistency limits			Shear Test		Specific Gravity	Density g/cc	IS Classification	Observed N Value	Corrected N Value	Safe Bearing Capacity T/m2	Core Recovery %	Rock Quality Designation %	Ultimate crushing strength (mpa)
				Gravel %	Sand %	Silt & Clay %	Liquid Limit %	Plastic Limit %	Plasticity Index %	Cohesion value C kg/cm2	Angle of Internal Friction									
03	1.00	SPT	-	-	-	-	-	-	-	-	-	-	-	SM	57	57.06	40.67	-	-	-
	2.00	SPT	16.4	20.45	77.3	2.3	NP	NP	NP	-	38	2.63	1.8	SM	>50	50.00	38.13	-	-	-
	3.00	SPT	9.33	25.33	68.7	6.0	NP	NP	NP	-	38	2.68	1.8	SM	>50	50.00	39.83	-	-	-
	4.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	41.53	-	-	-
	5.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	43.23	-	-	-
	6.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	44.93	-	-	-
	7.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	46.63	-	-	-
	8.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	48.33	-	-	-
	9.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	50.03	-	-	-
	10.00	DS	-	-	-	-	-	-	-	-	-	-	-	SM	>50	50.00	51.73	-	-	-

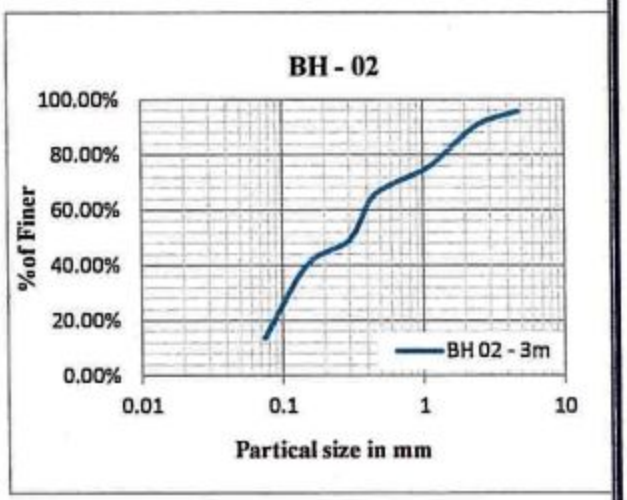
YUBE PRIME INFRA SALEM - 636 005

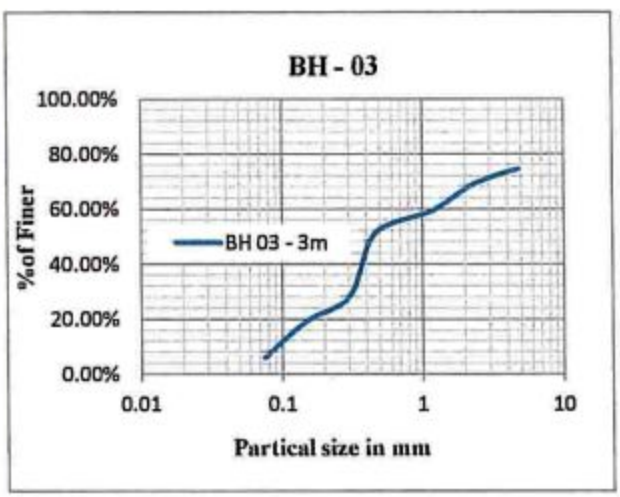
7 SIEVE ANALYSIS

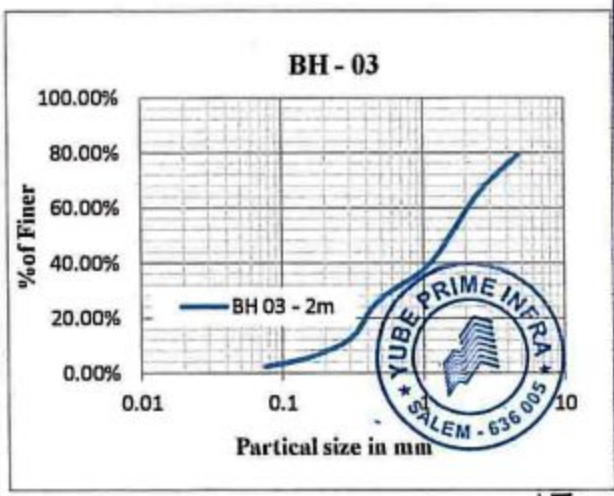












8 RECOMMENDATIONS

Type of Foundation

After critical review of the results, OPEN FOUNDATION in the form of ISOLATED FOOTING may be designed for this project.

Depth of foundation

The depth of foundation shall be placed at 3.00m from natural ground level.

Safe Bearing Capacity

The safe bearing capacity (SBC) is given below in the table.

SI NO	DEPTH (m)	ALLOWABLE SETTLEMENT	WATER TABLE	SBC t/m^2	SIZE OF FOOTING (m)
01	3.00	25mm	-	20.00	2.00 X 2.00
02	3.00	25mm	-	30.00	2.00 X 2.00
03	3.00	25mm	-	30.00	2.00 X 2.00

Permissible Settlement

The safe bearing capacity is recommended for a permissible settlement of 25mm.

Safety Measures

The top layer soil should be used for back filling.

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9 CERTIFICATION

FOR M/S. JMJ HOUSING, COIMBATORE, WE (YUBE PRIME INFRA) HAVE RECOMMENDED FOR THE PROPOSED CONSTRUCTION OF APARTMENT BUILDING AT S.F NO 155/2A, 155/2B & 155/2C AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT. HAVE GEOTECHNICAL INVESTIGATION WORK WAS CONDUCTED AT THE SITE ON 29-MAR-2021.AFTER ANALYSIS OF THE TEST REPORTS, IT IS RECOMMENDED THAT ISOLATED FOUNDATION TO BE ADOPTED FOR DESIGN PURPOSE WITH SOIL BEARING CAPACITY AND DEPTH OF FOUNDATION ARE GIVEN BELOW THE TABLE.

SOIL DATA AS PER COMPUTATION

SI NO	DEPTH (m)	ALLOWABLE SETTLEMENT	WATER TABLE	SBC t/m^2	SIZE OF FOOTING (m)
01	3.00	25mm	-	20.00	2.00 X 2.00
02	3.00	25mm	-	30.00	2.00 X 2.00
03	3.00	25mm	-	30.00	2.00 X 2.00

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FOR YUBE PRIME INFRA
Er. M.YUVARAJ, ME (GEOTECH), MIGS
(GEOTECHNICAL & STRUCTURAL CONSULTANT)

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10 SITE PHOTOS

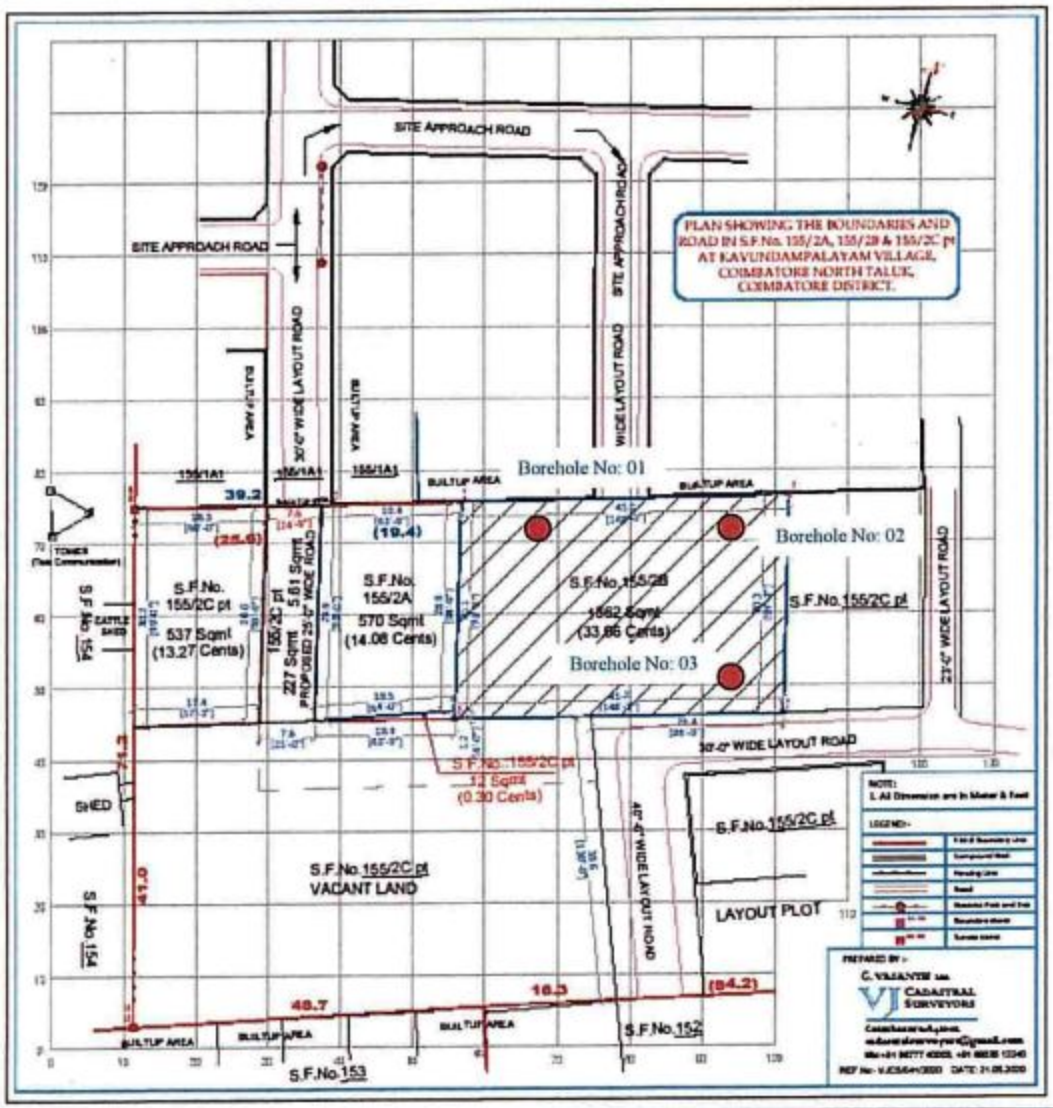








11 SITE PLAN


PLAN SHOWING THE BOUNDARIES AND ROAD IN S.F.No. 155/2A, 155/2B & 155/2C pt AT KAVUNDAMPALAYAM VILLAGE, COIMBATORE NORTH TALUK, COIMBATORE DISTRICT.
SITE APPROACH ROAD
30'-0" WIDE LAYOUT ROAD
23'-0" WIDE LAYOUT ROAD
Borehole No: 01
Borehole No: 02
Borehole No: 03
S.F.No. 155/2C pt
537 Sqmt (13.27 Cents)
S.F.No. 155/2A
570 Sqmt (14.08 Cents)
S.F.No.155/2B
1562 Sqmt (33.86 Cents)
227 Sqmt PROPOSED 25'-0" WIDE ROAD
S.F.No.155/2C pt
12 Sqmt (0.30 Cents)
S.F.No.155/2C pt VACANT LAND
LAYOUT PLOT
S.F.No.153
S.F.No.152
S.F.No.154
SHED
CATTLE SHED
BUILTUP AREA
39.2
(25.6)
(19.4)
41.0
48.7
16.3
(84.2)
PREPARED BY :-
C. VIJAYAN
VJ CADASTRAL SURVEYORS



our services doesn't just end there

Buying & Selling Services

Property Buying Assistance, Property Selling Support, Market Price Analysis, Owner Outreach and Negotiation.

Legal & Documentation Services

Certified EC, Patta, legal opinions, property verification, due diligence, litigation solutions, and legal heir certificates.

Property Development & Approvals

Building plan approvals, construction, layout and sub-division approvals, zone conversion, and occupancy certification.

NRI & Investment Services

Escrow Services, Rentals, Litigation Solutions, Property Valuation and Property Management

Compliance & Special Services

Demolition, Vaasthu checks, regularization of unauthorized buildings, and office space solutions.



empowering your real estate decisions with smart tools

Property Valuation & Cost Estimation

Guideline Value, Apartment Composite Value, Stamp Duty & Registration Fees Calculator, Building Value Calculator, Capital Gains Calculator, Area Unit Converter, Length Unit Converter

Ownership, Compliance & Legal Verification

Encumbrance Certificate (EC) Search, Find Your SRO, Temple Property Check, WAQF Property Check, View Patta/Chitta, View FMB, Patta Analyzer, EC Analyzer

Zoning & Land Use Analysis

Land Use Zone Finder, CRZ Zone Check, Aquifer Zone Check, CBA Check, EWS Check, Road Width Check, Proposed Road Widening Check, FSI Calculator

Property Documentation & Workflow Tools

Forms & Templates, Document Generator, Document Translation, Dictionary

Vastu & Property Management Tools

Home Vastu Shastra, Plot Vastu Shastra, Property Management

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