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SOIL INVESTIGATION REPORT

REPORT NO: -GEO-12/2022-23

DATE: -03.06.2022 & 04.06.2022

NAME OF THE WORK: -

GEOTECHNICAL INVESTIGATION FOR THE PROPOSED APARTMENT
BUILDING (STILT+5 FLOORS) AT OLD T. S NO: -192, NEW T.S.NO: -344/1,2,
&3, WARD NO: -8, WEST R.S. PURAM EXTENSION, PONNURANGAN
ROAD, R.S. PURAM, COIMBATORE DISTRICT.

REPORT SUBMITTED TO: -

Mr.SANTOSH SHARMA, Mr.DHIRAJ SHARMA & Mrs.BHAVANA SHARMA,
R.S. PURAM, COIMBATORE DISTRICT.



SUNSTAR FOUNDATION CONSULTANT

(Civil Engineering Consultant & Contractor)

Topographical Surveying, Soil Investigation, Pile Load Test, Plate Load Test, Rock Core Drilling,
Earth Resistivity & Laboratory Material Testing

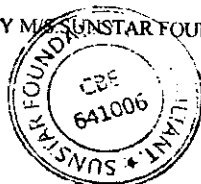
(An ISO 9001: 2015 Certified laboratory)

No: -15/16, First Floor, Kasthuribai 4th Street, Ganapathy, Coimbatore-641006.

Mob: -+91-8300404680, 8300404681, E-mail: -sunstarfoundationconsultant@gmail.com

www.sunstarfoundationconsultant.com

"GEOTECHNICAL INVESTIGATION CONDUCTED BY M/S SUNSTAR FOUNDATION CONSULTANT, COIMBATORE."



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Certified that this report contains pages 1 to 11 and further all the test has been conducted with high degree of reliability and accuracy.

Signatory of soil consultant M. Manandhakumar dated

Name of authorized signatory Er. MANANDHAKUMAR, BE, M.I.G.E.

Designation of signatory Project Engineer,
Sunstar Foundation Consultant.

Office Seal of Soil Consultant



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1.INTRODUCTION: -

Mr.Santosh Sharma,Mr.Dhiraj Sharma & Mrs.Bhavana Sharma,R.S. Puram, Coimbatore District has entrusted the work of conducting geotechnical investigations at structures locations pertaining to Subsoil Exploration for The Proposed Apartment Building (Stilt+5 Floors) at old T. S No: -192, New T.S.No: -344/1,2, &3, Ward No: -8, West R.S. Puram Extension, Ponnurangan Road, R.S. Puram, Coimbatore District was awarded to M/S.Sunstar Foundation Consultant. This report presents results of the geotechnical investigation along with foundation recommendations for the proposed structure.

1.1. SCOPE OF WORK: -

- Conducting Geotechnical Investigation along with suitable foundation recommendation for the proposed structure.
- Determine the soil profile at the site with ground water level observation.
- Conducting SPT's at regular depth intervals.
- Collecting Disturbed Soil (DS) Samples.
- Conducting relevant laboratory tests on Soil samples.
- Submitting report with Field observations, laboratory test results along with recommendations and safe bearing pressure at foundations level.

In this report, preliminary details of sub-soil layers encountered, brief account of the strength of the strata based on the Field Tests, discussion on foundation options and recommendations for foundations are presented.

2. SITE DISCRPTION: -

Based on boreholes information, sub surface profile for Subsoil Exploration for The Proposed Apartment Building (Stilt+5 Floors) at old T. S No: -192, New T.S.No: -344/1,2, &3, Ward No: -8, West R.S. Puram Extension, Ponnurangan Road, R.S. Puram, Coimbatore District. The area where the tests were conducted for the Borehole locations-lundulating level area with respect to surroundings.

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3. FIELD INVESTIGATION: -

3.1. Drilling in Overburden: -

Rotary drilling machines were mobilized and drilling operation was taken up. 100mm to 150mm diameter boreholes were made at pre-determined locations and the boreholes were advanced up-to varying depths. The drilling was commenced by driving a casing of suitable diameter in the upper layers of the bore holes. Drilling was carried out as per IS 1892 - 1979. In this method, boring is effected by the cutting action of a rotating bit that is kept in firm contact with the bottom of the hole. The bit is attached to the lower end of a hollow drill rod that is rotated by a suitable chuck. Drilling mud (usually Bentonite) is continuously forced down the hollow drill rods. The mud returning upwards through the annular space between the drill rods and the side of the hole brings the cutting to the surface. As stated earlier, the boreholes are terminated after sufficiently advancing in hard strata.

3.2. Standard Penetration Test: -

Standard Penetration Tests (SPT) were conducted as per IS 2131-1981. In a SPT test, a standard split spoon sampler is driven at the bottom of the borehole. The penetration resistance in terms of blows for 300mm penetration of the split spoon sampler is measured as 'N' Value. The blows are imparted by a standard weight of 63.5 kg falling through a height of 750 mm. The resistance is measured for 150 mm, 300 mm, and 450 mm penetrations. The resistance of first 150 mm is ignored since those blows are considered as seating blows due to the loosening of the strata. The resistance of next 300 mm is recorded as standard penetration value 'N'. If the sampler is driven less than 450 mm (total) then the penetration resistance is given for the last 300 mm of penetration. If the penetration depth is less than 300 mm and the blow count is more than 50 then the 'N' value is considered as 'Refusal' and actual penetration is recorded with the blows. These tests were conducted at close intervals of 1.50m

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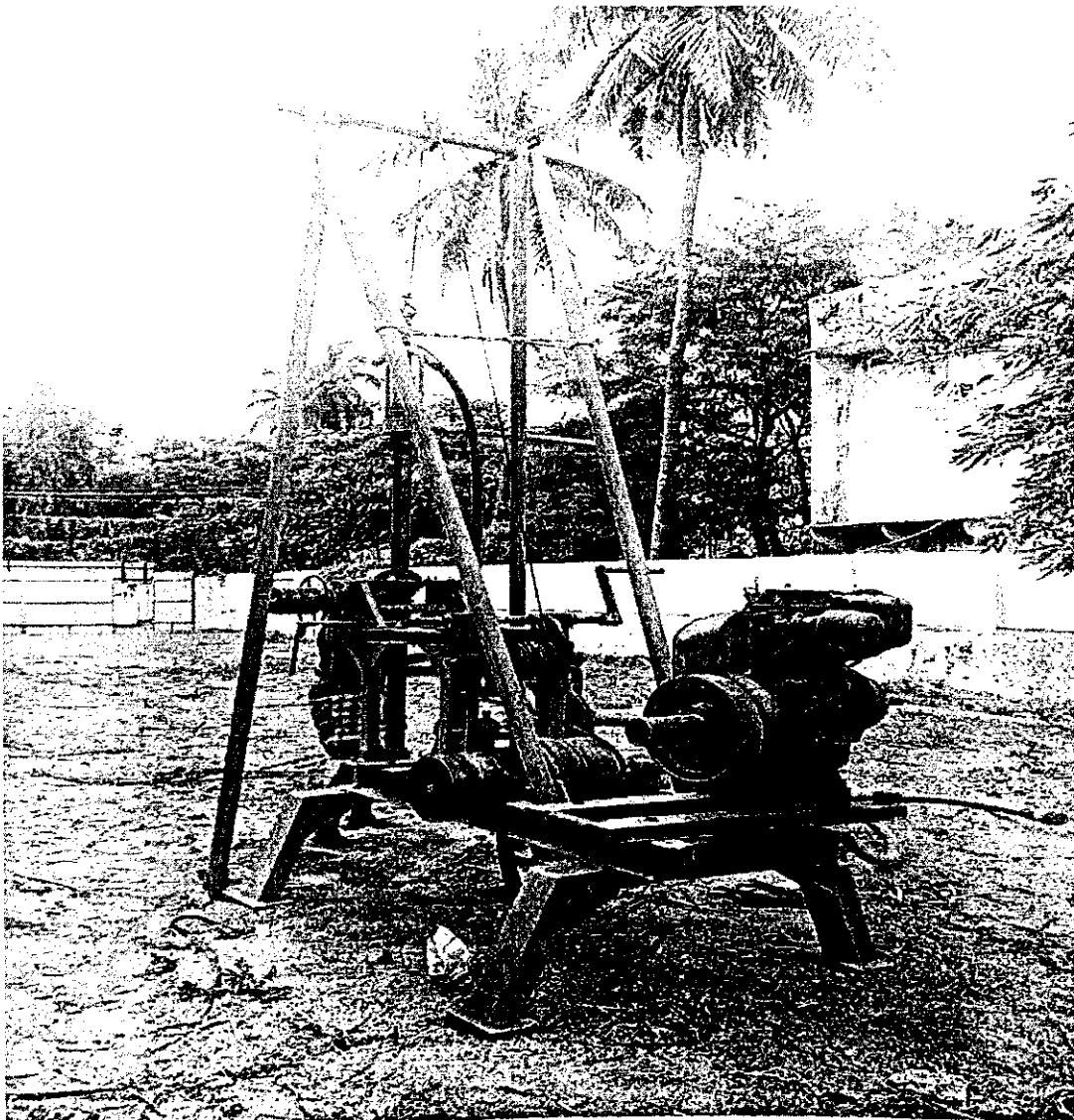
depth intervals at shallow depths. Disturbed Soil (DS) collected in the SPT sampler was preserved in polythene covers and transported to the laboratory. One more polythene cover was provided to prevent the loss of moisture during the transit period.

3.3. Collection of Samples: -

Soil samples were collected through Split spoon was done up to bore hole termination level.

4.FIELD INVESTIGATION PHOTOGRAPHS: -

BOREHOLE LOCATION



5.1. FIELD BORELOG DATA SHEET-1

NAME OF THE PROJECT: - "SUBSOIL EXPLORATION FOR THE PROPOSED APARTMENT BUILDING (STILT+5 FLOORS) AT OLD T. S NO: -192, NEW T.S.NO: -344/1,2, &3, WARD NO: -8, WEST R.S. PURAM EXTENSION, PONNURANGAN ROAD, R.S. PURAM, COIMBATORE DISTRICT".

TYPE OF BORING: -ROTARY	GPS COORDINATES	GROUND WATER TABLE: -Not Met
LOCATION: -1		DATE OF STARTED: -03.06.2022
BED LEVEL: -		DATE OF COMPLETED: -04.06.2022

Depth below ' NGL' (m)	Soil profile	Description of Strata	Thickness of sfratum (m)	Depth of samples collected (m)	SPT- 'N' Values Details				Type of samples collected	Relative Density/ Consistency	Graphical representation of SPT- 'N' Values				
					N1=15cm	N2=30cm	N3=45cm	N _{VALUE} = N2+N3			20	40	60	80	100
0.00		FILLING SOIL (SILTY SAND)	1.05	0.00	-	-	-	-	-	-					
1.05															
		GRAVELLY SAND	0.55												
1.60				1.50	50/8	-	-	>50	DS	V.Dense					
															
															
															
															
															
															
															
															
															
</															

6. LABORATORY INVESTIGATION:-

Following laboratory tests are conducted on the soil samples

- Natural Moisture Content
- Natural Wet Density
- Natural Dry Density
- Atterberg's Limit
- Sieve Analysis
- Specific Gravity
- Shear Test Parameters
- Consolidation Test



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6.1. LABORATORY TEST RESULTS-1

NAME OF THE PROJECT :-	"SUBSOIL EXPLORATION FOR THE PROPOSED APARTMENT BUILDING (STILLT 5 FLOORS) AT OLD T.S NO: -192, NEW T.S.NO: -344 I.2. &3, WARD NO:-8, WEST R.S. PURAM EXTENSION, PONNURANGAN ROAD, R.S. PURAM, COIMBATORE DISTRICT."
M/S SUNSTAR FOUNDATION	LOCATION :-

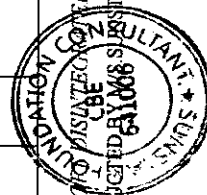
M/S.SUNSTAR FOUNDATION	LOCATION: -BH - I
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GROUND WATER TABLE: - Not Met

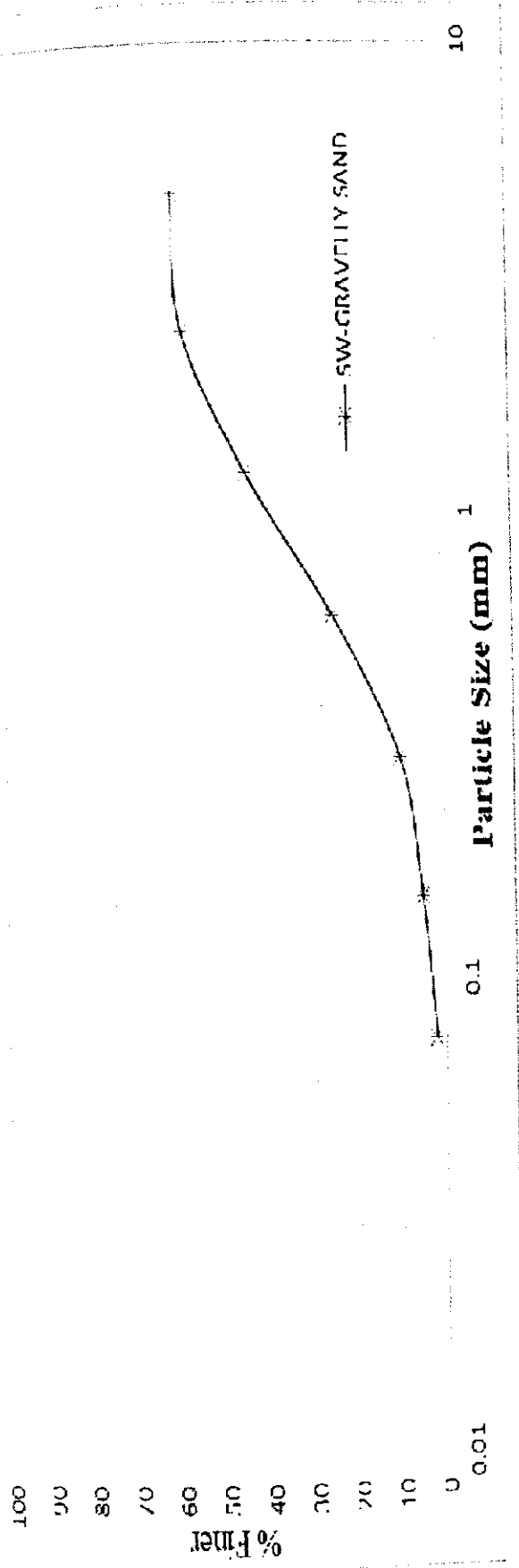
DATE OF LAB TESTING STARTED: -04.06.2022
DATE OF LAB TESTING COMPLETED: 08.06.2022

[illegible]

FS-FILLING SOIL, SW-GRAVELLY SAND, SDR-SOFT MISTREATED ROCK NP- NON-PLASTIC, DS- DISTURBED SAMPLE,
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7. GRAIN SIZE DISTRIBUTION CURVE: -



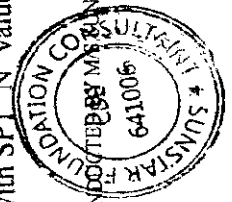
8. DISCUSSION ON FOUNDATION: -

8.1. Sub Surface Conditions: -

Subsurface profile at this site generally consists of filling soil (silty sand) then gravely sand overlying underlain by weathered rock. Groundwater accumulation in boreholes was monitored during and after completion of drilling activities. Groundwater was observed in boreholes at 3.10m depth from the existing ground level. Seasonal and annual fluctuations in ground water levels can be changed.

8.2. Engineering Appraisal: -

Based on field and laboratory results on the samples collected from investigation locations, the shear strength parameters such as c (cohesive strength) and ϕ (angle of internal friction) can be correlated with SPT 'N' values.



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8.3. Design Methodology: -

The safe bearing capacity of the soil can be calculated on shear failure criteria and settlement criteria and any foundation system has to satisfy the above said criteria. The former one depends on shear strength and the bearing capacity equation suggested by IS 6403 as follows. Net ultimate bearing capacity is given by

$$C N_c S_c d_c i_c + \gamma D (N_q - 1) S_q d_q i_q + 0.5 \gamma B N_{\gamma} S_{\gamma} d_{\gamma} i_{\gamma} W_{\gamma}$$

The safe bearing capacity is the maximum gross intensity of loading that the soil can carry safely without failing in shear which is obtained by the net ultimate bearing capacity divided by factor of safety (2.5 to 3.0)

The later one follows settlement criteria, that the intensity of the loading will cause within the permissible settlement is termed as allowable bearing pressure given by correcting the field SPT 'N' values.

1. **Correction due to dilatancy (N')**. This correction is based on the assumption that critical void ratio occurs at approximately $N = 15$, and in fine grained cohesionless soils, the coefficient of permeability is low that the excess pore water pressure developed by the driving impedes the penetration of split spoon sampler, thus increasing the N value.

$$N' = 15 + 0.5 (N_o - 15)$$

2. **Correction due to overburden pressure**

$$N = 0.77 N' \log_{10} (2000 / P' o)$$

$P' o$. effective over burden pressure should be greater than 25 kpa

Net allowable bearing pressure developed by Terzaghi and peck (1948)

$$Q_s = 3.5 (N_{cor} - 3) (B + 0.3 / 2B)^2 \times R_{w2} \times F_D + P$$

The Allowable Bearing Pressure of Soil = Net Allowable Bearing Pressure of Soil + $P' o$.

Where

N' = Corrected N value

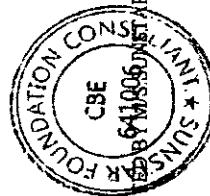
N'' = Corrected N value due to dilatancy

B = Size of the Footing = constant 1x1m

RW = Water Correction = constant 0.30m to 0.50m

FD = Depth of Factor

P = Over Burden Pressure



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9. BIS CODE & REFERENCES: -

S. NO	CODE NUMBERS	DESCRIPTION
1	IS 1888: 1982	Method of load tests on soils
2	IS 6403: 1981	Code of practice for Determination of Bearing capacity of foundation.
3	IS 1892: 1979	Code of practice for subsurface investigations for foundations.
4	IS 8009(Part-1&2) 1976	Code of practice for calculation of settlement of foundations
5	IS 2131: 1981	Method of standard penetration test for soils
6	IS 2132: 1986	Code of practice for thin-walled tube sampling of soils
7	IS 2720 Part 1 to Part 31 & IS 2720 Part 33 to Part 41	Methods of Test of Soils
8	IS 3043 : 1987	Code of practice of Earthing
9	IS 4464: 1985	Code of practice for presentation of drilling information and core description in foundation investigation
10	IS: 2911 (PART 1/ SEC 1-4) - 1979	Code of practice for pile design
11	IS 6935: 1973	Method of determination of water level in a borehole

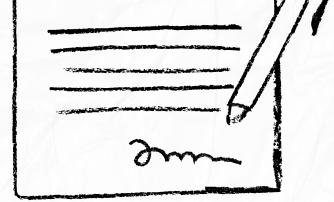
M. (2009)
Er. M. ANANDHAKUMAR, B.E., M.I.G.S
 Project Engineer,
 Sunstar Foundation Consultant.

20/07/2022
Dr. E. RAVI, ME., PhD (Soil Mechanics), F.I.E., F.I.G.S.,
 Chief Geotechnical Engineer,
 Sunstar Foundation Consultant.
Er. RAVI, ME., PhD (Soil Mechanics), F.I.E., F.I.G.S.,
 Chief Geotechnical Consultant,
 Sunstar Foundation Consultant.



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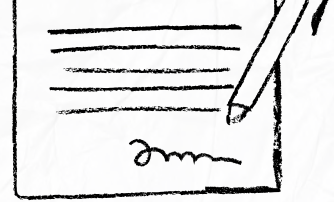
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