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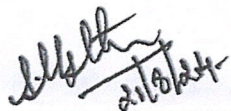
PROFESSIONAL REGISTRATION CERTIFICATE

This is to certify that the **Thiru. B. SRINIVASAN** has enrolled as **Registered Structural Engineer Grade-II (SE)** with Chennai Metropolitan Development Authority (CMDA) who has the qualification of **B.E., (Civil Engg.)** and experience in the relevant field for **18 years**.

He has been given the Registration No.**SE/GR-II/2019/08/152** Renewed on **21.08.2024**. He shall be required to comply with the duties and responsibilities prescribed by the Government of Tamil Nadu in Part-V, "Registration of Professionals", Tamil Nadu Combined Development and Building Rules, 2019 approved vide G.O.(Ms) No.18 MAWS (MA1) Dept. dated 04.02.2019.

This Registration is valid upto **20.08.2029**.




(A. MEGANATHAN)
Superintendent


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B.SRINIVASAN B.E AMIE.,

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STRUCTURAL STABILITY CERTIFICATE

STRUCTURAL STABILITY CERTIFICATE FOR THE PROPOSED CONSTRUCTION OF STILT FLOOR + 5 FLOORS (HEIGHT-18.30m) RESIDENTIAL BUILDING WITH 128 DWELLING UNITS AVAILING PREMIUM FSI WITH AFFORDABLE HOUSING BENEFIT WITH GYM AND INDOOR GAMES ROOM AT 5th FLOOR AT OTTIYAMBAKKAM MAIN ROAD, OTTIYAMBAKKAM, CHENNAI COMPRISED IN OLD S.Nos. 259/2A, 259/2B, 259/2C, 259/2D, 260/2A & 260/2B NEW S.Nos. 259/2A2, 259/2B2, 259/2C2, 259/2D2, 260/2A2 & 260/2B2 OF OTTIYAMBAKKAM VILLAGE, WITHIN THE LIMIT OF ST. THOMAS MOUNT PANCHAYAT UNION.

The Proposed Building referred to above is a framed structure with Stilt and 5 Upper Floors. The Structural Design and Drawing for the building are being prepared by me conforming to the relevant Bureau of Indian Standard viz. IS 456:2000 for Plain and Reinforced Concrete, IS 875(Part II) —1987 for Imposed Loads and IS 1893:2002(part 1) for Seismic Force. We certify that the building when constructed according to our Structural Designs and Drawings will be structurally safe and sound and fit for occupation.

1. The minimum grade of concrete is M30
2. The minimum width of Column at Stilt Floor is 300mm.


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DESIGN CALCULATION FOR THE PROPOSED CONSTRUCTION OF STILT FLOOR + 5 FLOORS (HEIGHT- 18.30m) RESIDENTIAL BUILDING WITH 128 DWELLING UNITS AVAILING PREMIUM FSI WITH AFFORDABLE HOUSING BENEFIT WITH GYM AND INDOOR GAMES ROOM AT 5th FLOOR AT OTTIYAMBAKKAM MAIN ROAD, OTTIYAMBAKKAM, CHENNAI COMPRISED IN OLD S.Nos. 259/2A, 259/2B, 259/2C, 259/2D, 260/2A & 260/2B NEW S.Nos. 259/2A2, 259/2B2, 259/2C2, 259/2D2, 260/2A2 & 260/2B2 OF OTTIYAMBAKKAM VILLAGE, WITHIN THE LIMIT OF ST. THOMAS MOUNT PANCHAYAT UNION.

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1.0 INTRODUCTION

1.1 *General Project Information*

Project Title

RESIDENTIAL BUILDING CONSIST OF STILT AND 5 UPPER FLOORS.

Location of Project

OTTIYAMBAKKAM MAIN ROAD, OTTIYAMBAKKAM, CHENNAI

Project Description

RCC Framed Building consisting of Stilt and Five Upper floors.

1.2 **Purpose of Basis of Design Document**

This document details the input data, principles, methods and performance criteria to be used in the structural engineering design of the Project.

The document serves as a plan for the basis of justification and calculations associated with the design and are a repository for all relevant structural information, including references and computer programs pertaining to the project.

The contents of this brief is based on the drawings available at time of design and actual structural working drawings may change to suit latest architectural design at time of construction.

1.3. **Structural Design**

The main considerations followed for the design of structure are:

- (a) Structural safety and stability.
- (b) Demands of aesthetics conceived by the architect.
- (c) Availability of material, equipment and expertise.
- (d) Constructability and ease of maintenance.
- (e) Durability.
- (f) Economy.
- (g) Recyclability.


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2.0 STRUCTURAL SYSTEM

This section provides an overview of the selected structural system and the structural concept employed for the Project. The stability of each building against imposed lateral loads, namely seismic loads /wind load is provided by RCC columns.

RCC BUILDING - REINFORCED CONCRETE STRUCTURE

2.1.1 Foundations

The design options for Foundation shall be worked out based on the Soil test report shared by client. The foundations are to comprise of Individual Footing.

2.1.2 Superstructure - Gravity System

The load resisting system is to be framed utilising RCC slabs and beams with columns. Frames are classified as Ordinary Moment-Resisting Frames (OMRF).

2.1.3 Expansion joint

As the total length of the building is found to be not more than normal condition it is found that expansion joint is not necessary.

2.1.4

Seismic loads and earthquake resistant design shall be as per IS 1893(Part-1). Earthquake forces shall not be considered to act at the same time as wind forces. Direction of earthquake loading and combination thereof shall be as per clause 6.3.2 of IS 1893(Part-1).

Zone	: III (For Chennai)
Zone factor	: 0.16
Importance factor (I)	: 1.5
Response reduction factor (R)	: 5.0
Avg. response acceleration coefficients (Sa/g)	: As per soil condition

The building is designed by static analysis


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3.0 DESIGN CODES, STANDARDS AND REFERENCE DOCUMENT

3.1 Codes, Standards and Commentaries

- a) IS 456 - 2000 Plain and Reinforced Concrete structures
- b) IS 875 - 1987 Design Loads
- c) IS 3370 Part I, Part II and Part IV Concrete Structures for storage of liquids
- d) SP 16 Design aids for Reinforced Concrete structures
- e) SP 34 Concrete Reinforced & Detailing
- f) IS 1786 Specification for high strength deformed bars
- g) National Building Code of India 1983
- h) IS 13920 -2016 for Seismic detailing

3.2 Design References and Guides

- a) Reinforced Concrete Design -SP 16 BIS handbook
- b) Reinforced Concrete Designers Handbook

3.3 Specification used

All the buildings have been designed as framed structures in accordance with the relevant Indian Codes of Practice for civil works i.e. IS: 456-2000. Seismic detailing is followed for the entire building wherever found to be necessary.

4.0 LOADSCRITERIA

The structure is analyzed for all possible combinations of loads i.e. gravity loads(dead and live loads). It should be noted that no extra provision has been made in the design of foundation and vertical structural elements for additional floors.

4.1 MATERIAL DEAD LOADS

Item	Density (kN/m³)
Dense Concrete	25
Steelwork	78.5
Soil	18.0
Water	10.0
200 thk AAC Block with 8 mm plaster internal walls	8.0


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4.2 MINIMUM IMPOSED LOADS

RESIDENTIAL BUILDINGS (KN)

a) All rooms and Kitchen	2.0
b) Toilets and bathrooms	2.0
c) Corridors, passages, lobbies and staircases including fire escapes - as per the floor serviced but not less than	3.0

IMPOSED LOADS ON ROOFS

Roofs of buildings used for promenade or incidental to assembly purposes shall be designed for the appropriate imposed floor loads given in Table for the occupancy.

4.3 WIND LOADING

The building structure is of normal high so wind load does not dominate the result of the analysis.

4.4 FIRE RESISTANCE:

Cover to main rebars will be provided as per Table 16 in BIS 456 for fire rating of 1.0 hours.

4.5 EXPOSURE:

The Building will be exposed to Moderate environment as per Table 5 in BIS456.


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5.0 **REINFORCED CONCRETE STRENGTHS.**

5.1 Concrete design grades are specified cube strengths to BIS

Elements	Concrete Grade Fck (N/sqmm)
Columns/Shear Walls	25
Foundations	25
Beams / Slabs	25
Lean Concrete	10

5.2 **REINFORCEMENT**

Reinforcement Yield Strengths used in the design of concrete elements:

Deformed Bars	500 MPa (D)
---------------	-------------

6.0 **GEOTECHNICAL**

Geotechnical Model and recommendations are based on report shared by the Client

The recommendations for the proposed structure (Residential building .comprising of an Stilt and Five upper floors)are given below:

- a) Type of Foundations : Individual footing
- b) Depth of foundations : 4.5 to 5.5mt from the existing ground level
- c) Allowable bearing pressure : 30t/m²
- d) Settlement considered for calculations: <50mm.


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Design of TYP FOOTING

From Analysis the Load to the Footing are 400 KN from a square column 300x750 mm (with 25mm bar). M25 Grade of Concrete and Fe 500 steel is used and safe bearing capacity of soil is taken as 100KN/sqm as per soil report.

$$\text{Maximum Load on Column} = 1100 \text{ KN.}$$

$$\text{Add 10\% self weight of Footing} = 110 \text{ KN.}$$

$$\text{Total} = 1210 \text{ KN.}$$

$$\text{Area of Footing required } A = \frac{1210 \text{ K}}{300 \text{ KN/m}^2} = 4.03 \text{ m}^2.$$

$$\text{Providing footing size} = 2.00 \times 2.00 \text{ m.}$$

$$\text{Ultimate Soil reaction} = P/A = \frac{1.5 \times 1210}{2.0 \text{ m} \times 2.0 \text{ m}}$$

$$= 453 \text{ KN/m}^2$$

$$= 0.453 \text{ N/sqmm.}$$

Depth of footing for shear.

$$d = 0.453 \times 2000 \times (950 - d)$$

$$= 860700 - 906d.$$

$$\text{Assume } P_t = 0.12 \zeta_c = 0.29 \text{ N/sqmm.}$$

shear resistance

$$\begin{aligned} V_{c1} &= 0.29 \times \\ &2000 \times d \\ &= 580d \end{aligned}$$

$$V_{u1} \leq V_{c1}$$

$$= 860700 - 906d \leq 580d$$

$$d \geq 600 \text{ mm Providing } d=660 \text{ D}=700.$$


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Flextural moment

$$M_u = w l^2 / 2$$

$$= 303 \times 4 \times 1.65 \times 1.65 / 2 = 528 \text{ KNm} =$$

$$528148000 \text{ Nmm. } M_u / b d^2 =$$

$$528148000 / 3500 \times 640 \times 640 = 0.61 \text{ Mpa.}$$

$$P_t = 1.9\% \text{ Ast} = P_t \times b \times d / 100 = 1.9 \times 1000 \times 460 / 100 = 861 \text{ sqmm/m.}$$

Using 12 mm rod for Footing. Spacing of bar = $1000 \times 112 / 861 = 120 \text{ mm}$.

Use 12mm rod @ 100mm/c both ways for Footing.

And Design of other Footings calculated in similar way

Design of TYP Column

From Analysis the Column Axial load is found to be 980 KN.

The size of the column is taken as

230mmx750mm. Grade of Concrete M30

and Grade of steel Fe500 Cover = 40mm.

Ht of Column = 3mt. $F_y = 500 \text{ N/sqmm}$ &

$F_{ck} = 25$

Effective Length ratio 0.65 as per IS 456:2000 (both end fixed)

$$L_{eff1} = 0.65 \times 3 = 1.95, L_{eff2} = 0.65 \times 3 = 1.95$$

Check for Slenderness

$$L_{eff1}/D = 6.5 \quad L_{eff1}/b = 6.5 \quad L_{eff2}/D = 6.5 \quad L_{eff2}/b$$

$$= 4.3 \text{ All Columns are short about both axis Size}$$

of Column and check e_{min}

$$e_{min} = L_{eff}/500 + D/30 = 1950/500 + 300/30$$

$$= 13.9 \text{ Find } M_{min} = 0.031 \times 980 =$$

$$30.38 \text{ KNm.}$$

Design Axial Load $P_u = 980 \text{ KN}$.

Design Moment $M_x = M_{min} = 30.38 \text{ KNm}$.

Designing the column as an axially loaded column with uni-axial moment

Calculation of Area of Steel.

Design Axial Load $P_u = 480 \text{ KN}$.

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Design Moment $M_x = P_u \times e_{min} = 30.38 \text{ kNm}$.

$$d' = 40 + 20/2 = 50 \text{ mm}$$

$$d'/D = 50/300 = 0.217$$

$$M_u / F_{ck} b D^2 = 30.38 \times 1.5 \times 10^6$$

$$/ 30 \times 230 \times 750 \times 750 = 0.03 \quad P_u / F_{ck} b d =$$

$$980 \times 1.5 \times 10^3 / 30 \times 230 \times 750 = 0.43 \quad \text{From SP 16}$$

$$\text{Chart 44, } p / F_{ck} = 0.089$$

$$A_{st} = 0.089 \times 30 \times 230 \times 750 / 100 = 2208 \text{ sqmm.}$$

Required 10 Nos of 25mm tor rods.

For Distributing even arrangement of rod on all 4 sides of column Also Lateral ties 8 tor rods @ 150c/c are to be Provided. And Design of other Column calculated in similar way.

Design of TYP Beam

Beam size is taken as 230mm x 600mm.

Simply supported beam Effective span is 4.18mt

From analysis the Beam moment are as follows

Moment Top near support (left) is -224 kNm.

Moment Top near support (right) is -209 kNm.

Moment Bottom @ Mid span is +281 kNm.

Area of steel @ Top near support (left)

$$M_u = 0.87 A_{st} f_y d [1 - (A_{st} f_y / b d f_{ck})]$$

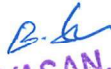
$$-224000000 = 0.87 \times A_{st} \times 500 \times 560 [1 - (A_{st} \times 500 / 230 \times 560 \times 25)]$$

$$-224000000 = 243600 A_{st} - 37.82 A_{st}^2$$

$$A_{st} = 416 \text{ sqmm.}$$

Providing 3-16# @ Top near Left Support

Area of steel @ Top near support (right)


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$$M_u = 0.87 A_{st} f_y d [1 - (A_{st} f_y / b d f_{ck})]$$

$$-209000000 = 0.87 \times A_{st} \times 500 \times 560 [1 - (A_{st} \times 500 / 230 \times 560 \times 25)]$$

$$-209000000 = 243600 A_{st} - 37.82 A_{st}^2$$

$$A_{st} = 466 \text{ sqmm.}$$

Providing 3-16# @ Top near Left Support

Area of steel @ Bottom near mid span

$$M_u = 0.87 A_{st} f_y d [1 - (A_{st} f_y / b d f_{ck})]$$

$$281000000 = 0.87 \times A_{st} \times 500 \times 560 [1 - (A_{st} \times 500 / 230 \times 560 \times 25)]$$

$$281000000 = 243600 A_{st} - 37.82 A_{st}^2$$

$$A_{st} = 505 \text{ sqmm.}$$

Providing 3-16# & 3-16# @ Bottom of Mid span

Also Stirrups 8 tor rods @ 150c/c are to be Provided.

And Design of other Beam calculated in similar way.

Design of TYP Slab

Effective span of Slab is 3.89mt x 7.2mt with beam support of 230mm spaced @ 3890mmc/c

Taking M30 Grade Concrete and Fe500 Grade steel.

$$\text{Thickness of slab} = 3890/35 = 111\text{mm.}$$

Slab thickness is taken as 125mm as the building is used for Residence purpose.

$$\text{Effective depth} = d = 125 - 20 - 6 = 99\text{mm.}$$


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Self wt of slab = $0.125 \times 25 = 3.125 \text{KN/sqm}$ Finish = 1.5KN/sqm .

Total Dead Load = 5.25KN/sqm .

Live load for Residence building Lobby Corridor etc = 2KN/sqm .

Partition Load on slab is taken as 2KN/sqm .

Total Load = $2+2+5.25 = 9.25 \text{KN/sqm}$.

Factored Load $W = 1.5 \times 9.25 = 13.875 \text{KN/sqm}$.

Coefficient in Shorter direction = 0.1149 .

Shorter Span Positive moment = $0.1149 \times 13.875 \times 4.5 \times 4.5 =$

46.24 KNm . $M_u \text{ limit} = 0.138 f_{ck} b d d = 0.138 \times 25 \times 1000 \times 97$

$\times 97 = 53 \text{ KNm}$ Since M_u is less than $M_u \text{ limit}$ the section is

under reinforced. $M_u = 0.87 A_{st} f_y d [1 - (A_{st} f_y / b d f_{ck})]$

$46240000 = 0.87 \times A_{st} \times 500 \times 124 [1 - (A_{st} \times 500 / 1000 \times 97 \times 25)]$

$43240000 = 53940 A_{st} - 8.7 A_{st}^2$

$A_{st} = 645 \text{ sqmm}$.

Provide 10 Tor @ 100mm c/c .

Coefficient in Longer direction = 0.034 .

Longer Span Positive moment = $0.034 \times 19.875 \times 4.5 \times 4.5 =$

13.68 KN.m $M_u \text{ limit} = 0.138 f_{ck} b d d = 0.138 \times 25 \times 1000 \times$

$97 \times 97 = 53 \text{ KNm}$ Since M_u is less than $M_u \text{ limit}$ the section is

under reinforced. $M_u = 0.87 A_{st} f_y d [1 - (A_{st} f_y / b d f_{ck})]$

$13680000 = 0.87 \times A_{st} \times 500 \times 124 [1 - (A_{st} \times 500 / 1000 \times 97 \times 25)]$

$13680000 = 53940 A_{st} - 8.7 A_{st}^2$

$A_{st} = 264 \text{ sqmm}$.

Provide 10 Tor @ 150mm c/c .


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Check for Shear.

$$\text{Span load} = 19.875 \times 4.5 = 89 \text{ KN} \quad V = 89/2 = 44.7 \text{ KN.}$$

$$\text{Actual Shear stress } V = 44700/1000 \times 97 = 0.36 \text{ N/sqmm} < T_c(\text{min}).$$

From IS 456 Table 19 (Using $P_t = 0.65\%$) Design shear strength = 0.574 N/sqmm .

$$\text{Max Allowable shear stress} = 3.95 \text{ N/sqmm.}$$

Check for Deflection.

$$\text{Actual Span to depth ratio} = 4500/97 =$$

36.2 Calculation of Allowable Span to depth ratio.

$$f_s = 0.58 f_y (\text{Area of c.s of steel required} / \text{Area of c.s of steel provided})$$

$$= 0.58 \times 500(945/1120) = 244$$

$$\text{N/sqmm. } K = 1/[0.225 + 0.003f_s -$$

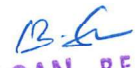
$$0.625 \log(1/p_t)].$$

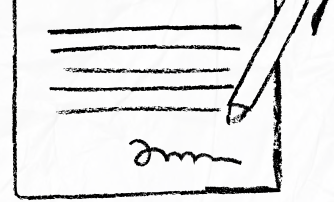
$$= 1/[0.225 + 0.003 \times 265 - 0.625 \log(1/0.65)].$$

$$= 1.3$$

$$\text{Allowable Span to depth ratio} = 30 \times K = 30 \times 1.3 = 39 < \text{Actual ratio.}$$

And Design of other Slab calculated in similar way.


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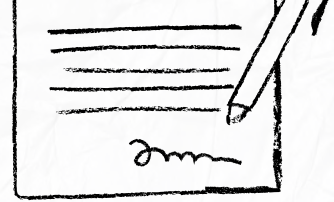
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