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*
*      STAAD.Pro CONNECT Edition
*      Version  22.12.00.142
*      Proprietary Program of
*      Bentley Systems, Inc.
*      Date=    JAN  6, 2026
*      Time=    11:45: 7
*
*  Licensed to: United Consultanta
*****
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1. STAAD SPACE
INPUT FILE: D:\MARIKRISHNAN\PROJECTS\STABILITY CERTIFICATE\AJ -3696- SUBSHIKA HOUSING - KODAMBAKKAM\AJ.. .STD
2. START JOB INFORMATION
3. ENGINEER DATE 12/26/2025
4. JOB NO AJ 3749
5. JOB NAME THE PROPOSED CONSTRUCTION OF STILT FLOOR + 5 FLOORS RESIDENTIAL -
6. BUILDING AT DOOR NO.378/26 PART, ARCOT ROAD, KODAMBAKKAM, CHENNAI COMPRISED -
7. IN OLD T.S.NO.73/2, NEW T.S.NO.73/1(AS PER DOCUMENT) AND T.S.NO.73/3 & -
8. 73/4(AS PER PATTI), BLOCK NO.29 OF PULIYUR VILLAGE, EGMORE TALUK WITHIN THE -
9. LIMITS OF GREATER CHENNAI CORPORATION
10. END JOB INFORMATION
11. INPUT WIDTH 79
12. SET NL 50
13. UNIT METER KN
14. JOINT COORDINATES
15. 1 0 0 0; 19 0 -3 0; 223 0 3.5 0; 378 3.125 -3 0; 422 0 6.75 0; 432 0 10 0
16. 439 0 13.25 0; 475 0 16.5 0; 482 0 19.75 0; 484 3.125 0 0; 485 3.125 6.75 0
17. 486 3.125 10 0; 487 3.125 13.25 0; 488 3.125 3.5 0; 489 3.125 16.5 0
18. 490 3.125 19.75 0; 491 6.25 -3 0; 492 6.25 0 0; 493 6.25 6.75 0; 494 6.25 10 0
19. 495 6.25 13.25 0; 496 6.25 3.5 0; 497 6.25 16.5 0; 498 6.25 19.75 0
20. 499 9.336 -3 0; 500 9.336 0 0; 501 9.336 6.75 0; 502 9.336 10 0
21. 503 9.336 13.25 0; 504 9.336 3.5 0; 505 9.336 16.5 0; 506 9.336 19.75 0
22. 507 11.393 -3 0; 508 11.393 0 0; 509 11.393 6.75 0; 510 11.393 10 0
23. 511 11.393 13.25 0; 512 11.393 3.5 0; 513 11.393 16.5 0; 514 11.393 19.75 0
24. 515 16.078 -3 0; 516 16.078 0 0; 517 16.078 6.75 0; 518 16.078 10 0
25. 519 16.078 13.25 0; 520 16.078 3.5 0; 521 16.078 16.5 0; 522 16.078 19.75 0
26. MEMBER INCIDENCES
27. 1 1 484; 25 19 1; 43 223 488; 98 1 223; 127 223 422; 173 422 485; 234 422 432
28. 254 432 486; 315 432 439; 335 439 487; 347 378 484; 348 484 488; 349 488 485
29. 350 485 486; 351 486 487; 407 439 475; 408 487 489; 414 475 489; 420 475 482
30. 421 489 490; 427 482 490; 429 484 492; 430 485 493; 431 486 494; 432 487 495
31. 433 488 496; 434 489 497; 435 490 498; 436 491 492; 437 492 496; 438 496 493
32. 439 493 494; 440 494 495; 441 495 497; 442 497 498; 444 492 500; 445 493 501
33. 446 494 502; 447 495 503; 448 496 504; 449 497 505; 450 498 506; 451 499 500
34. 452 500 504; 453 504 501; 454 501 502; 455 502 503; 456 503 505; 457 505 506
35. 459 500 508; 460 501 509; 461 502 510; 462 503 511; 463 504 512; 464 505 513
36. 465 506 514; 466 507 508; 467 508 512; 468 512 509; 469 509 510; 470 510 511
37. 471 511 513; 472 513 514; 474 508 516; 475 509 517; 476 510 518; 477 511 519
38. 478 512 520; 479 513 521; 480 514 522; 481 515 516; 482 516 520; 483 520 517
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39. 484 517 518; 485 518 519; 486 519 521; 487 521 522
 40. DEFINE MATERIAL START
 41. ISOTROPIC CONCRETE
 42. E 2.17185E+07
 43. POISSON 0.17
 44. DENSITY 23.5616
 45. ALPHA 1E-05
 46. DAMP 0.05
 47. G 9.28139E+06
 48. TYPE CONCRETE
 49. STRENGTH FCU 27579
 50. END DEFINE MATERIAL
 51. MEMBER PROPERTY
 52. 1 43 173 254 335 414 427 429 TO 435 444 TO 450 459 TO 465 474 TO 479 -
 53. 480 PRIS YD 0.508 ZD 0.23
 54. 451 TO 457 466 TO 472 PRIS YD 0.45 ZD 0.23
 55. 481 TO 487 PRIS YD 0.535 ZD 0.23
 56. 25 98 127 234 315 347 TO 351 407 408 420 421 436 TO 442 PRIS YD 0.23 ZD 0.54
 57. CONSTANTS
 58. MATERIAL CONCRETE ALL
 59. SUPPORTS
 60. 19 378 491 499 507 515 FIXED
 61. DEFINE WIND LOAD

*** NOTE: If any floor diaphragm is present in the model Wind Load definition
 should be defined after Floor Diaphragm definition. Otherwise wind
 load generation may be unsuccessful during analysis.

62. TYPE 1 WIND 1
 63. <! STAAD PRO GENERATED DATA DO NOT MODIFY !!!
 64. IS875-2015:PARAMS 0 0 0 16.000 0.000 5.000 0 CHENNAI 50.000 0 1.000 0 5 -
 65. 1.050 1.050 1.050 1.090 1.096 1.000 2 1.000 0.002 0 10.453 16.558 19.000 0 0 -
 66. 0 1.000 0 0 0.200 1.000 0 1.000 1.000 1.000
 67. !> END GENERATED DATA BLOCK
 68. INT 1.654 1.654 1.654 1.782 1.802 HEIG 0 5 10 15 16
 69. EXP 1 JOINT 1 19 223 378 422 432 439 491 499 507 515
 70. DEFINE IS1893 2016 LOAD
 71. ZONE 0.16 RF 2.5 I 1 SS 1 ST 1 DM 0.05 PX 0.5 PZ 0.5 DX 7.62 DZ 13.72
 72. MEMBER WEIGHT
 73. 25 98 127 234 315 347 TO 351 436 TO 440 451 TO 455 466 TO 470 481 TO 484 -
 74. 485 UNI -15 0 0
 75. LOAD 1 LOADTYPE SEISMIC-H TITLE EQ X
 76. 1893 LOAD X 1
 77. PERFORM ANALYSIS


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STAAD SPACE

-- PAGE NO. 3

PROBLEM STATISTICS -----

NUMBER OF JOINTS	48	NUMBER OF MEMBERS	77
NUMBER OF PLATES	0	NUMBER OF SOLIDS	0
NUMBER OF SURFACES	0	NUMBER OF SUPPORTS	6

Using 64-bit analysis engine.

SOLVER USED IS THE IN-CORE ADVANCED MATH SOLVER

TOTAL PRIMARY LOAD CASES =	1,	TOTAL DEGREES OF FREEDOM =	252
TOTAL LOAD COMBINATION CASES =	0	SO FAR.	

***NOTE: Equivalent Static Analysis should preferably be performed for regular structures with approximate natural time period less than 0.4s and regular structures with height less than 15m in Seismic Zone II. Ref Cl.6.4.3 and 7.6 BASE SHEAR AND TIME PERIOD IN X

```
*****
* UNITS - KN    METE                                *
* TIME PERIOD FOR X 1893 LOADING =    0.50000 SEC    *
* SA/G PER 1893=    2.000, LOAD FACTOR= 1.000        *
* VB PER 1893=    0.0640 X    1462.50=    93.60 KN    *
* VB Act Based on Clause 7.2.1 =    93.60 KN          *
* VB Min based on Clause 7.2.2 =    16.09 KN          *
*                                                     *
*****
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78. CHANGE
79. LOAD 2 LOADTYPE SEISMIC-H TITLE EQ -X
80. 1893 LOAD X -1.
81. PERFORM ANALYSIS

***NOTE: Equivalent Static Analysis should preferably be performed for regular structures with approximate natural time period less than 0.4s and regular structures with height less than 15m in Seismic Zone II. Ref Cl.6.4.3 and 7.6 BASE SHEAR AND TIME PERIOD IN X

```
*****
* UNITS - KN    METE                                *
* TIME PERIOD FOR X 1893 LOADING =    0.50000 SEC    *
* SA/G PER 1893=    2.000, LOAD FACTOR=-1.000        *
* VB PER 1893=    0.0640 X    1462.50=    -93.60 KN    *
* VB Act Based on Clause 7.2.1 =    -93.60 KN          *
* VB Min based on Clause 7.2.2 =    -16.09 KN          *
*                                                     *
*****
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82. CHANGE
 83. LOAD 3 LOADTYPE SEISMIC-H TITLE EQ Z
 84. 1893 LOAD Z 1
 85. PERFORM ANALYSIS

***NOTE: Equivalent Static Analysis should preferably be performed for regular structures with approximate natural time period less than 0.4s and regular structures with height less than 15m in Seismic Zone II. Ref Cl.6.4.3 and 7.6
 BASE SHEAR AND TIME PERIOD IN Z

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*****
* UNITS - KN    METE
* TIME PERIOD FOR Z 1893 LOADING = 0.50000 SEC
* SA/G PER 1893= 2.000, LOAD FACTOR= 1.000
* VB PER 1893= 0.0640 X 1462.50= 93.60 KN
* VB Act Based on Clause 7.2.1 = 93.60 KN
* VB Min based on Clause 7.2.2 = 16.09 KN
*
```

86. CHANGE
 87. LOAD 4 LOADTYPE SEISMIC-H TITLE EQ -Z
 88. 1893 LOAD Z -1.
 89. PERFORM ANALYSIS

***NOTE: Equivalent Static Analysis should preferably be performed for regular structures with approximate natural time period less than 0.4s and regular structures with height less than 15m in Seismic Zone II. Ref Cl.6.4.3 and 7.6
 BASE SHEAR AND TIME PERIOD IN Z

```
*****
* UNITS - KN    METE
* TIME PERIOD FOR Z 1893 LOADING = 0.50000 SEC
* SA/G PER 1893= 2.000, LOAD FACTOR=-1.000
* VB PER 1893= 0.0640 X 1462.50= -93.60 KN
* VB Act Based on Clause 7.2.1 = -93.60 KN
* VB Min based on Clause 7.2.2 = -16.09 KN
*
```

90. CHANGE
 91. LOAD 5 LOADTYPE DEAD TITLE DL
 92. SELFWEIGHT Y -1 LIST ALL
 93. MEMBER LOAD
 94. 1 43 173 254 335 414 429 TO 434 444 TO 449 459 TO 464 474 TO 479 UNI GY -16


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95. 427 435 450 465 480 UNI GY -8
 96. MEMBER LOAD
 97. 1 43 173 254 335 414 427 429 TO 435 444 TO 450 459 TO 465 474 TO 480 UNI GY -2
 98. LOAD 6 LOADTYPE LIVE TITLE LL
 99. MEMBER LOAD
 100. 1 43 173 254 335 414 427 429 TO 435 444 TO 450 459 TO 465 474 TO 480 UNI GY -3
 101. LOAD 7 LOADTYPE WIND TITLE WL Z
 102. WIND LOAD Z 1 TYPE 1
 103. LOAD 8 LOADTYPE WIND TITLE WL -Z
 104. WIND LOAD Z -1 TYPE 1
 105. LOAD COMB 9 1 (DL+LL)
 106. 5 1.0 6 1.0
 107. LOAD COMB 10 1.5 (DL+LL)
 108. 5 1.5 6 1.5
 109. LOAD COMB 11 1.2 (DL+LL+EQX)
 110. 1 1.2 5 1.2 6 1.2
 111. LOAD COMB 12 1.2 (DL+LL-EQX)
 112. 2 1.2 5 1.2 6 1.2
 113. LOAD COMB 13 1.2 (DL+LL+EQZ)
 114. 3 1.2 5 1.2 6 1.2
 115. LOAD COMB 14 1.2 (DL+LL-EQZ)
 116. 4 1.2 5 1.2 6 1.2
 117. LOAD COMB 15 1.2 (DL+LL+WLZ)
 118. 5 1.2 6 1.2 7 1.2
 119. LOAD COMB 16 1.2 (DL+LL-WLZ)
 120. 5 1.2 6 1.2 8 1.2
 121. LOAD COMB 17 1.5 (DL+EQX)
 122. 1 1.5 5 1.5
 123. LOAD COMB 18 1.5 (DL-EQX)
 124. 2 1.5 5 1.5
 125. LOAD COMB 19 1.5 (DL+EQZ)
 126. 3 1.5 5 1.5
 127. LOAD COMB 20 1.5 (DL-EQZ)
 128. 4 1.5 5 1.5
 129. LOAD COMB 21 1.5 (DL+WLZ)
 130. 5 1.5 7 1.5
 131. LOAD COMB 22 1.5 (DL-WLZ)
 132. 5 1.5 8 1.5
 133. PERFORM ANALYSIS

 134. START CONCRETE DESIGN
 CONCRETE DESIGN
 135. CODE INDIAN
 136. ENSH 1 ALL
 137. FC 25000 MEMB 1 43 173 254 335 414 427 429 TO 435 444 TO 450 459 TO 465 474 -
 138. 475 TO 480


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06 January 2026 10:58

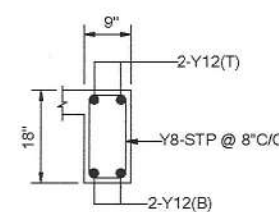
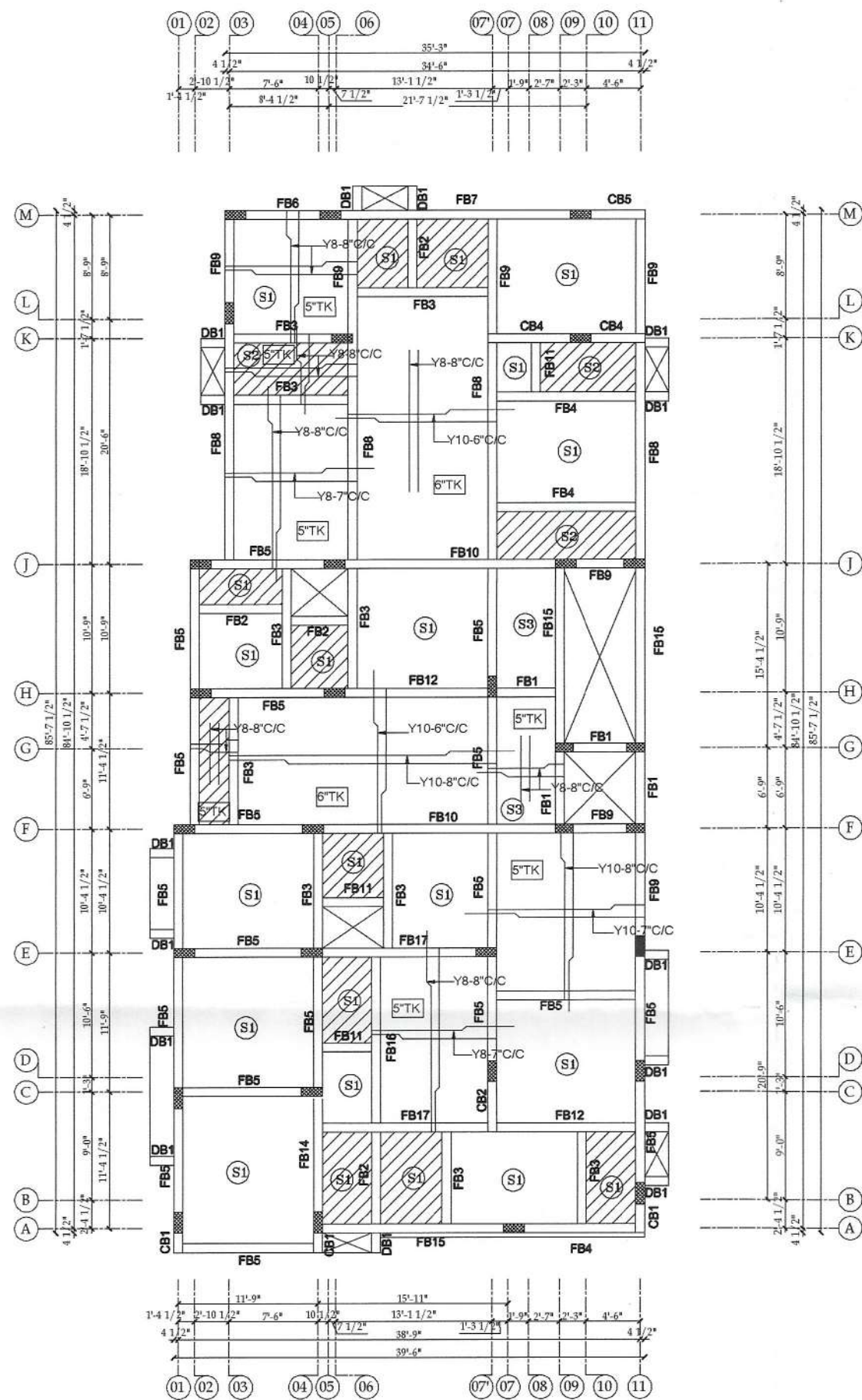
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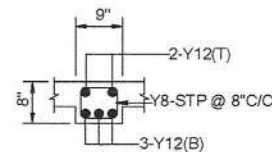
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143. 475 TO 480

DM

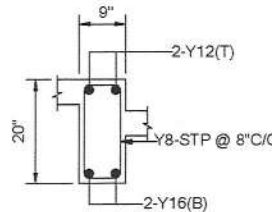
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REG No. SE/GR-10
REG No. RSE10



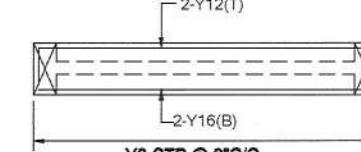
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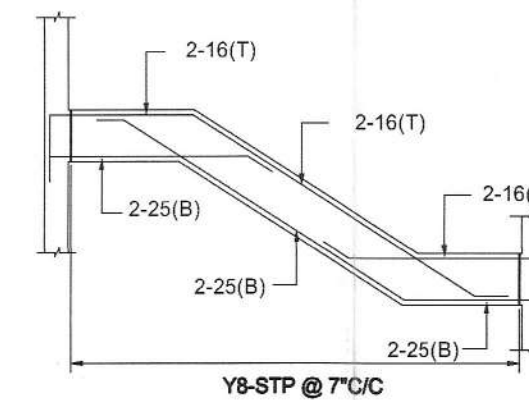
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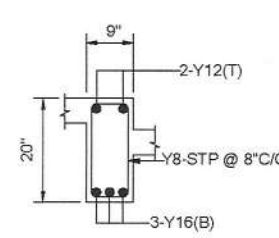
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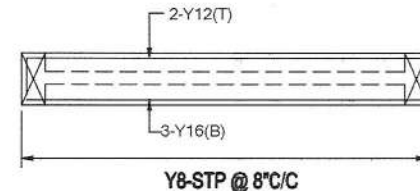
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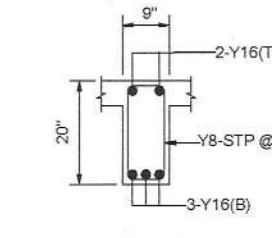
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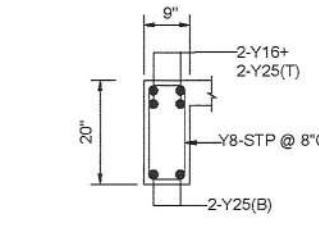
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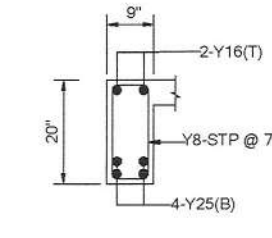
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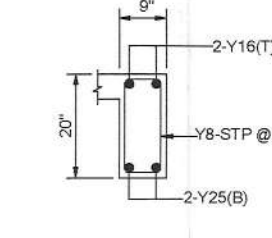
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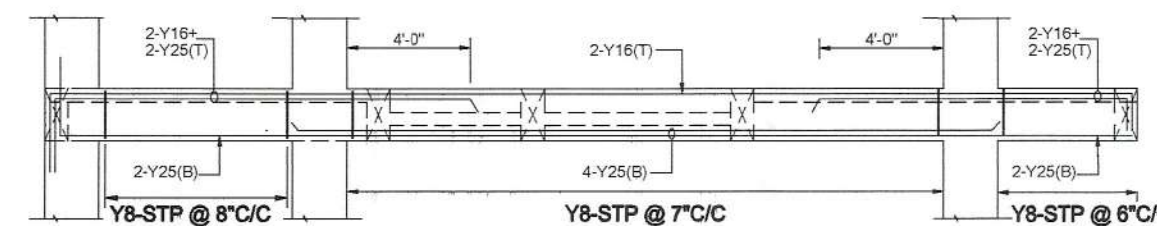
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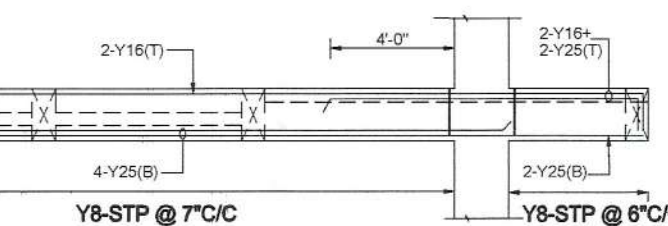
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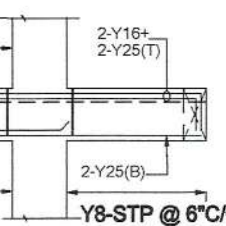
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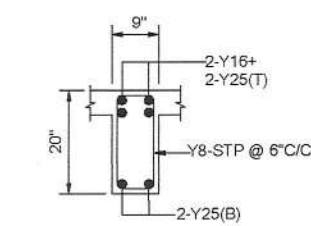
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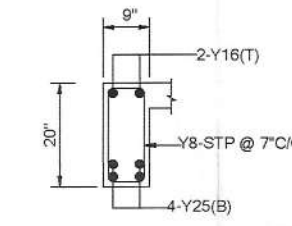
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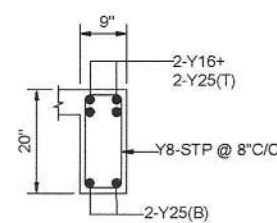
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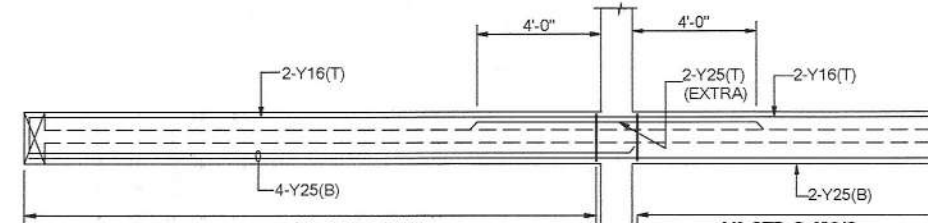
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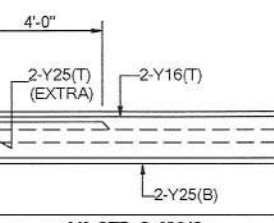
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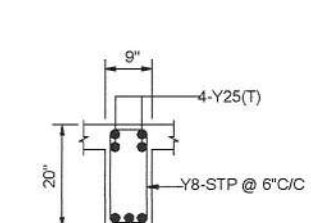
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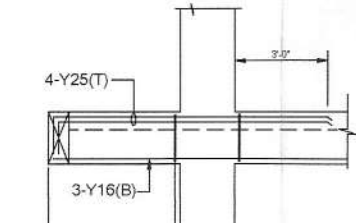
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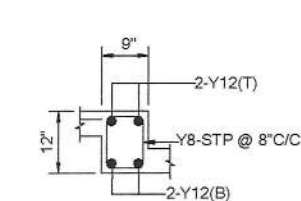
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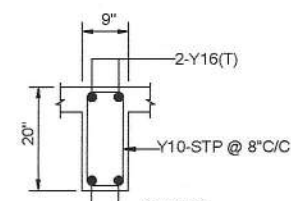
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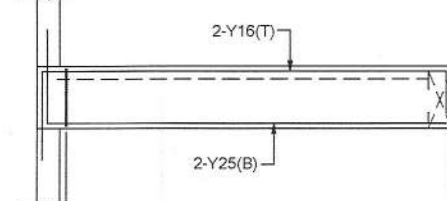
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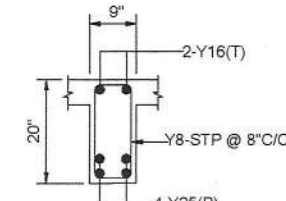
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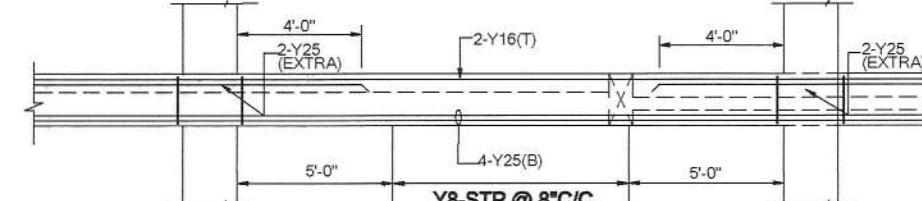
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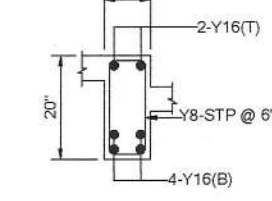
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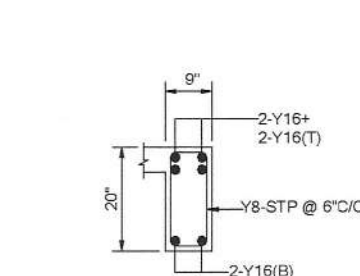
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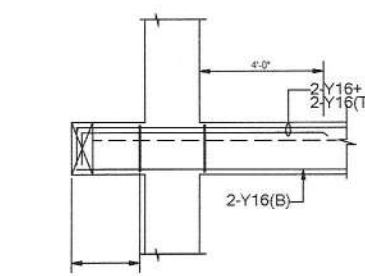
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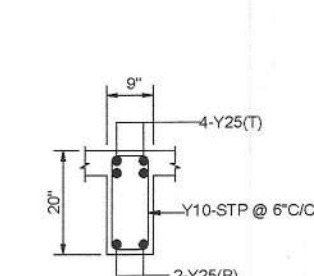
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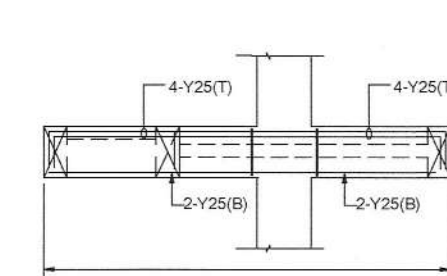
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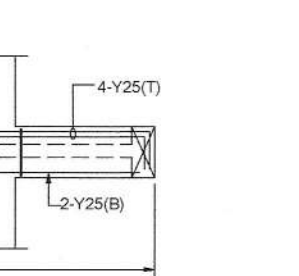
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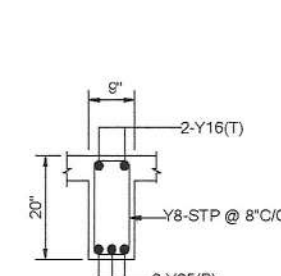
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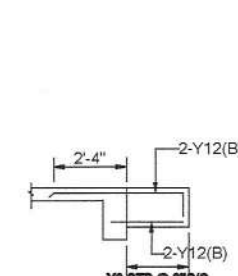
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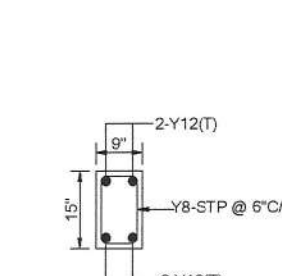
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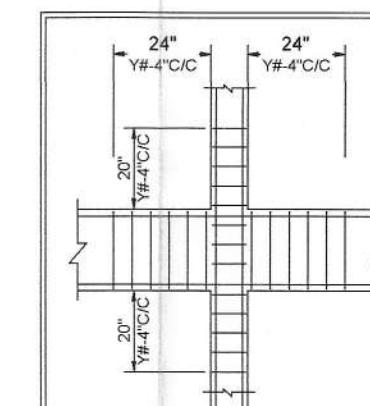
C.S OF FB17



L.S OF DB1



C.S OF DB1



DENOTES 8 OR 10 OR 12MM WHICH EVER MAXIMUM DIA OF THE STRIUPS BAR USED IN BEAM OR COLUMN

NOTES

G+5

- REFER ARCH. DWG. FOR CENTRE LINE DIMENSIONS.
- CONCRETE MIX SHALL BE M35 FOR PILE WITH 50mm MINIMUM CEMENT CONTENT OF 400KG/M³
- CONCRETE MIX SHALL BE M30 FOR COLUMN & M25 FOR ALL OTHER R.C.C WORKS
- ALL REINFORCEMENTS ARE OF Fe500 GRADE.
- CLEAR COVER TO MAIN REINFORCEMENT SHALL BE

(i) COLUMN	40mm
(ii) BEAM	30mm
(iii) SLAB	25mm
(iv) R.C.C WALL	20mm
- LAP LENGTH SHALL BE 50 x DIA OF THE BAR

R.N	Description of Revision	Date	Revised By

STRUCTURAL CONSULTANT

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 41/A, BEACH ROAD, DOOR NO. A1
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PROJECT TITLE

The Proposed construction of Stilt Floor+5 Floors Residential Building at Door No.378/26 part, Arcot Road, Kodambakkam, Chennai comprised in Old T.S.No.73/2, New T.S.No.73/1(as per document) and T.S.No.73/3 & 73/4(as per patta), Block No.29 of Puliur Village, Egmore Taluk within the Limits of Greater Chennai Corporation

DRAWING TITLE

TYPICAL FLOOR ROOF DETAIL

DWG NO

AJ -3698- SUBSHIKA HOUSING - KODAMBAKKAM

DSND Dr.Alex Jacob

DRN BIBIN

DATE 09.12.2025

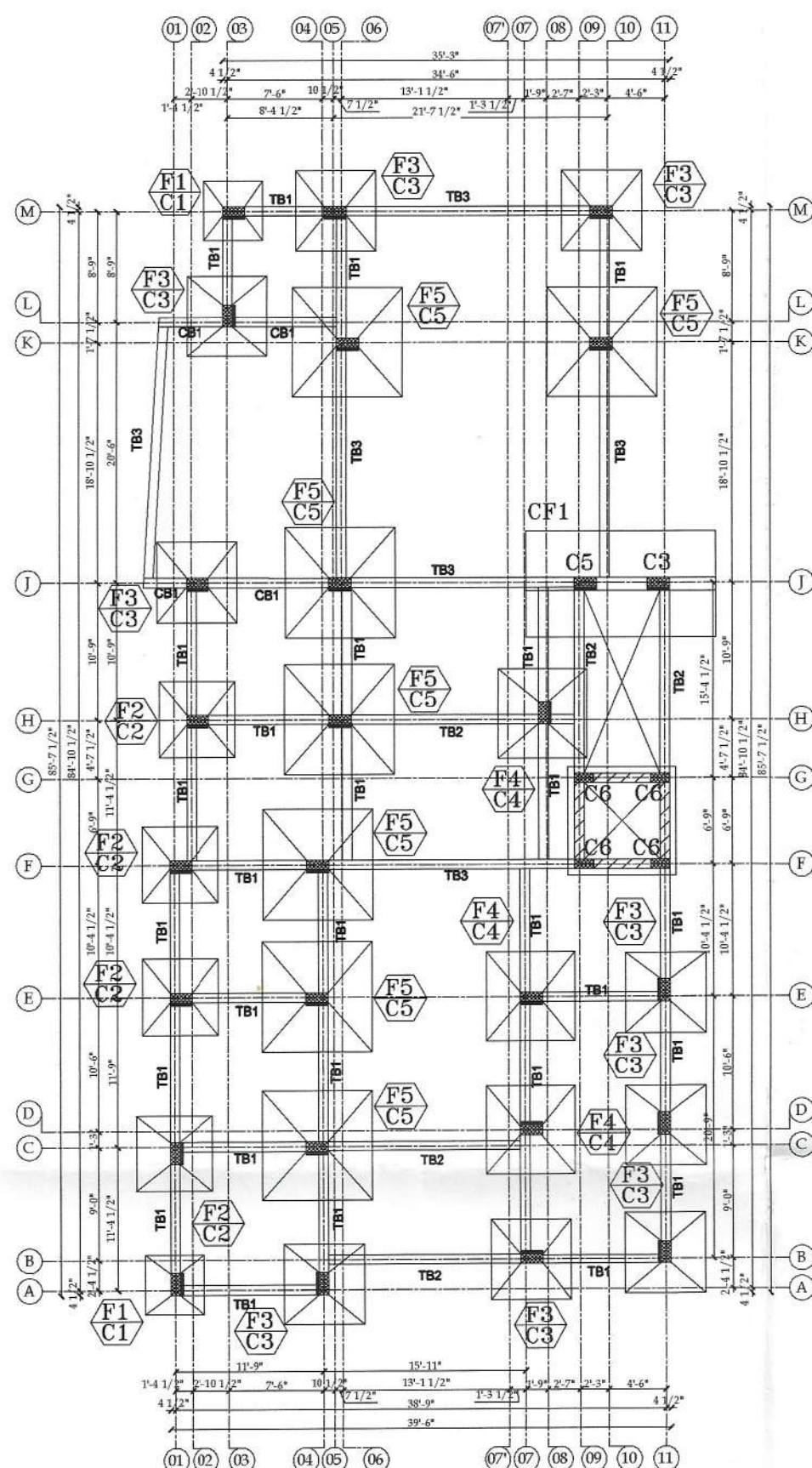
SCALE 1 : 8"

FILE NAME E:\BIBIN\AJ-3697-SUBSHIKA HOUSING-KODAMBAKKAM-ROOF DETAIL

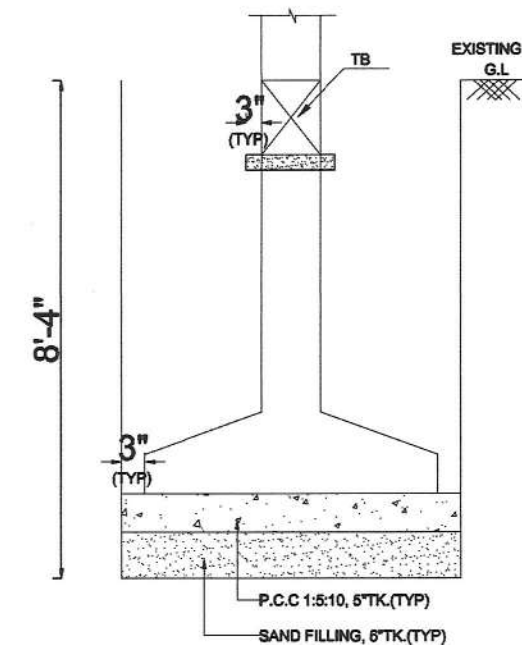
Dr. ALEX JACOB
 B.E., M.Tech(Str), Ph.D(Hon),
 M.I.E. (Aust), M.I.E. (Ind)
 STRUCTURAL ENGINEER
 70, BEACH ROAD, KALAKSHETRA COLONY,
 Chennai - 600 090.
 REG No. SE/GR-I/24/02/001
 REG No. RSE100392025

NOTES

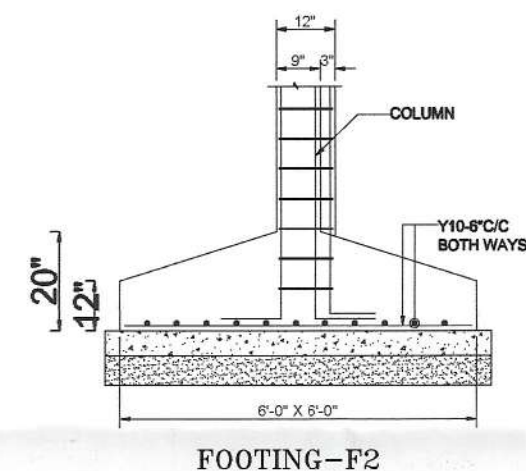
- REFER ARCH. DWG. FOR CENTRE LINE DIMENSIONS.
- CONCRETE MIX SHALL BE M30 FOR COLUMN & M25 FOR ALL OTHER R.C.C WORKS.
- ALL REINFORCEMENTS ARE OF Fe500 GRADE.
- CLEAR COVER TO MAIN REINFORCEMENT SHALL BE
 - (i) FOOTING 50mm
 - (ii) COLUMN 40mm
 - (iii) TIE BEAM 25mm
 - (iv) R.C.C.WALL 25mm
- LAP LENGTH SHALL BE 50 x DIA OF THE BAR



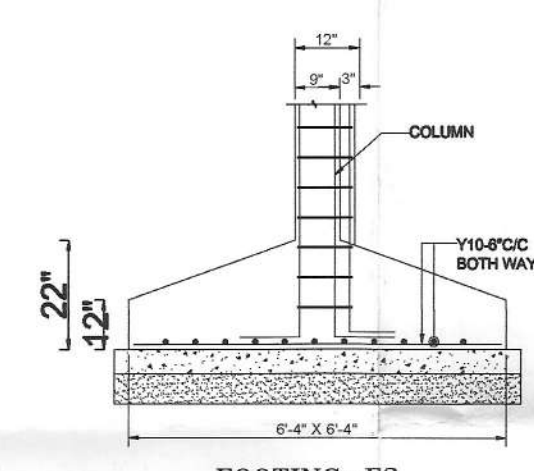
COL-NO	SIZE	REINFORCEMENT		SECTION	SHAPE OF TIES
		MAIN REINF.	TIES		
COL-C1	9" x 21"	4-Y16(c)+ 2-Y12(d)	Y8-TIES @ 8"C/C		
COL-C2	9" x 21"	6-Y16(c)	Y8-TIES @ 8"C/C		
COL-C3	9" x 21"	4-Y20(b)+ 2-Y16(c)	Y8-TIES @ 8"C/C		
COL-C4	9" x 21"	6-Y20(b)	Y8-TIES @ 8"C/C		
COL-C5	9" x 21"	6-Y25(a)	Y8-TIES @ 8"C/C		
COL-C6 (LIFT WELL)	9" x 18"	4-Y16(c)+ 2-Y12(d)	Y8-TIES @ 8"C/C		



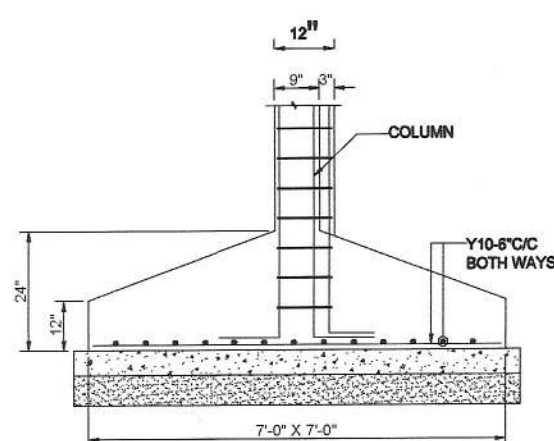
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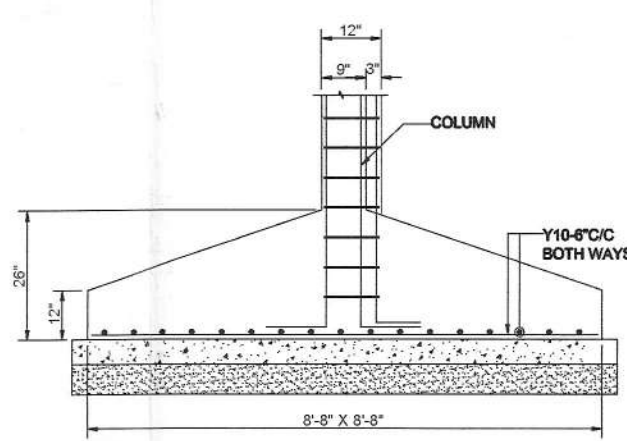
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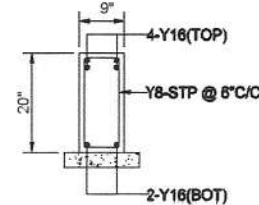
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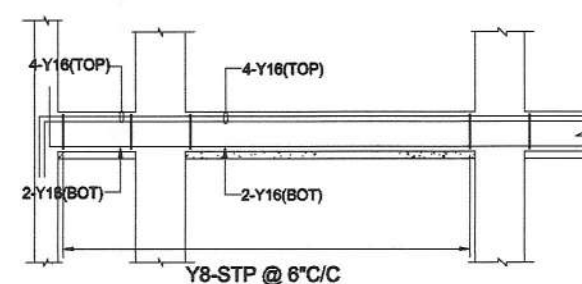
FOOTING-F4



FOOTING-F5

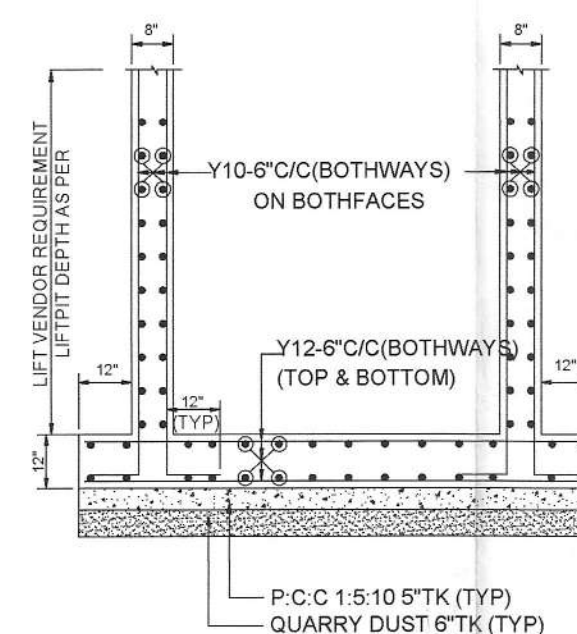


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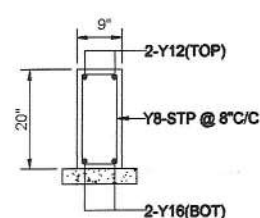


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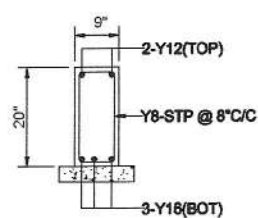
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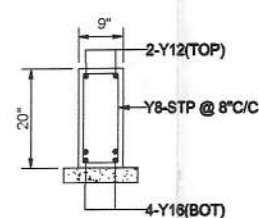
LIFTWELL SECTION



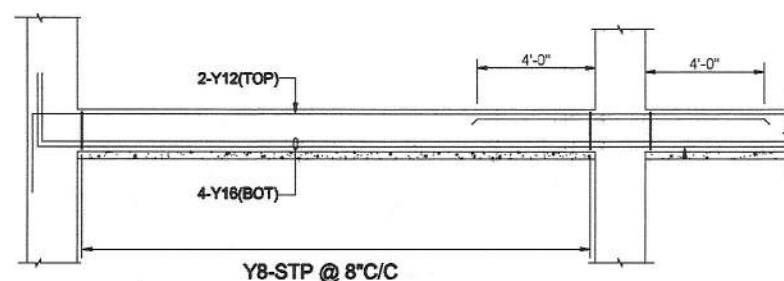
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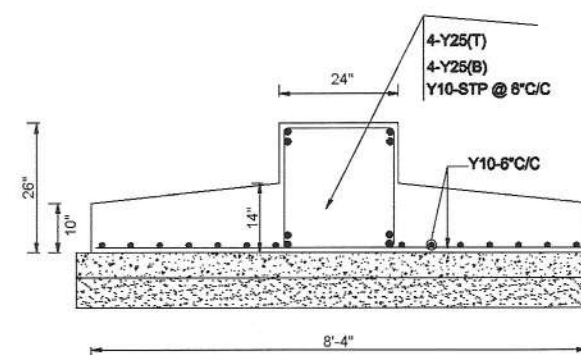
C.S. OF TB2



C.S. OF TB3



L.S. OF TB3



FOOTING-CF1

R.N Description of Revision Date Revised By

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FOUNDATION DETAIL

DWG NO

AJ -3696- SUBSHIKA HOUSING - KODAMBAKKAM

DSND Dr.Alex Jacob

DRN GOPIKA

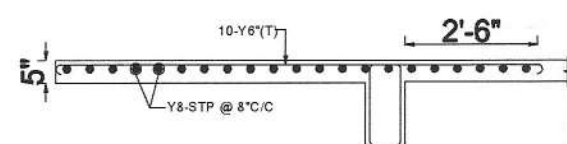
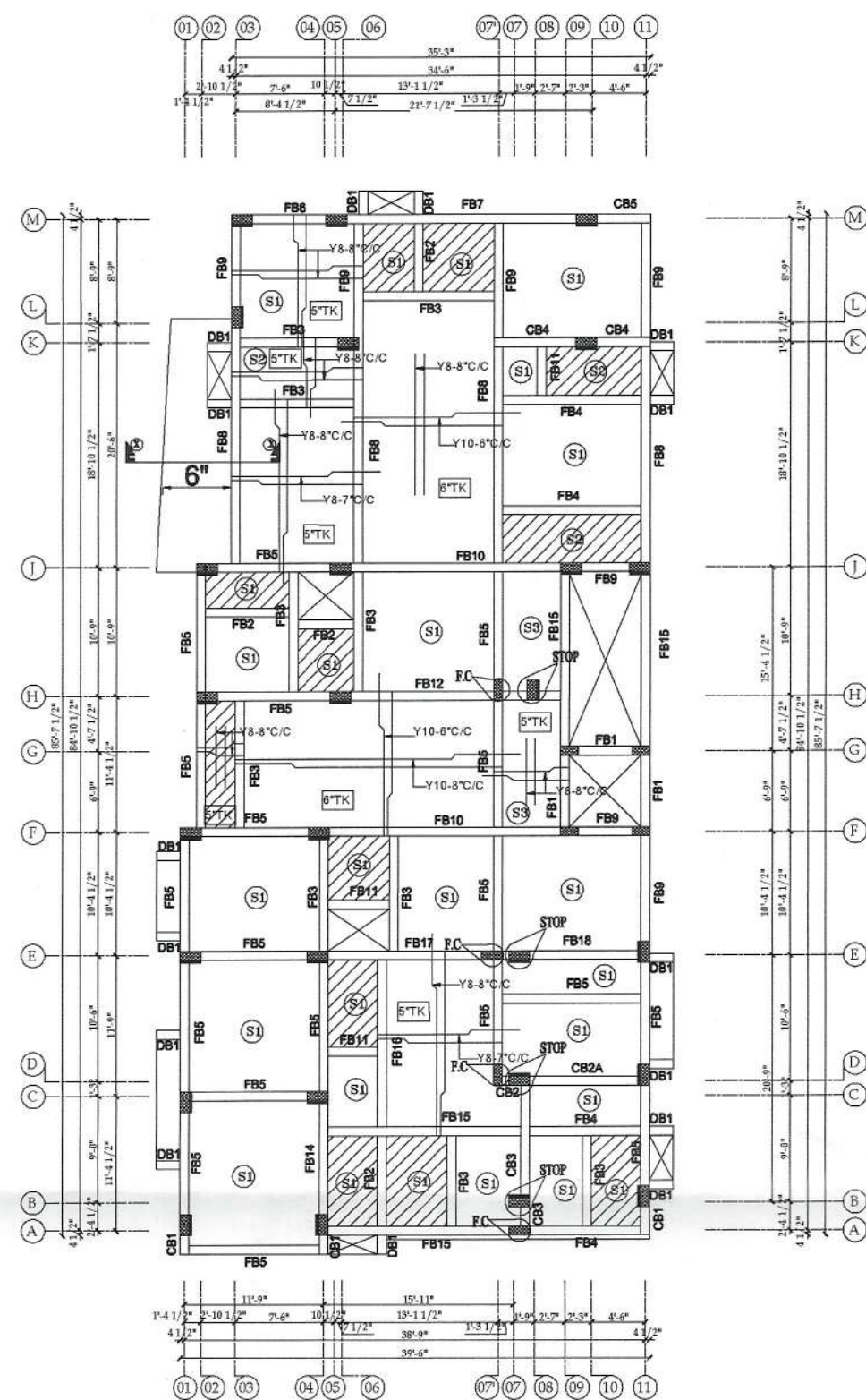
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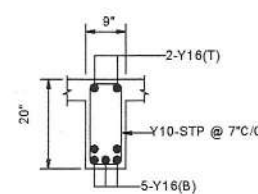
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E:\GOPIKA\AJ-3697-SUBSHIKA HOUSING - KODAMBAKKAM - FOUNDATION DETAIL

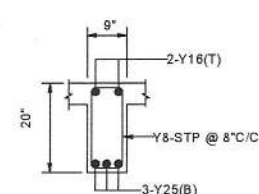
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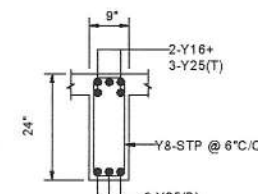
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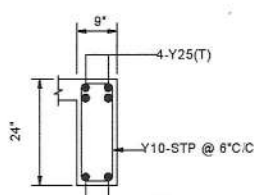
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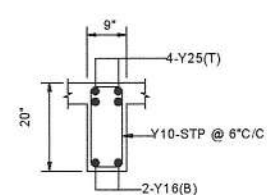
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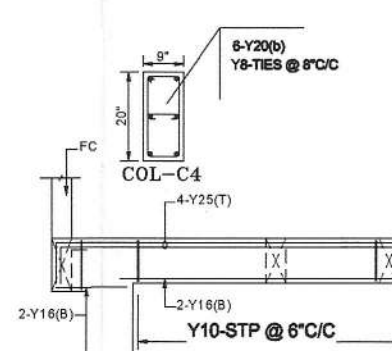
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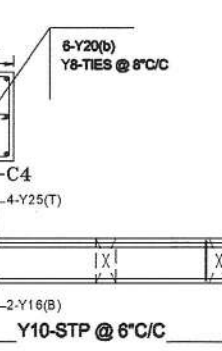
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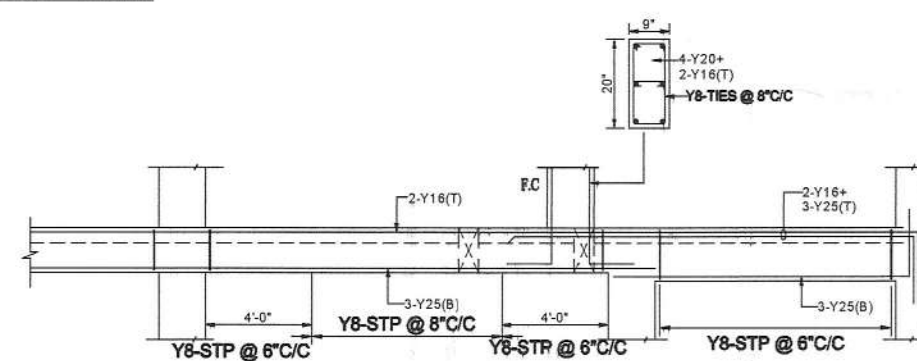
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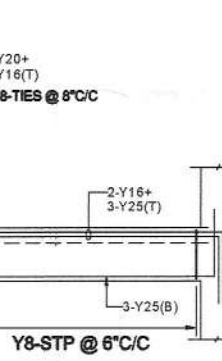
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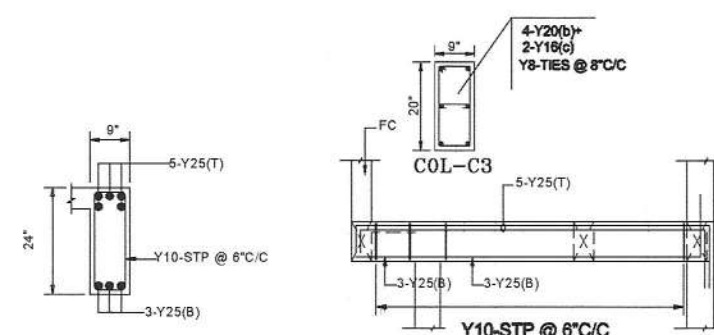
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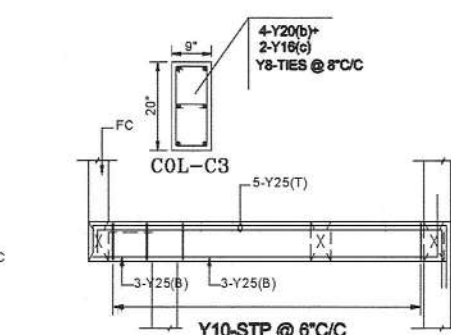
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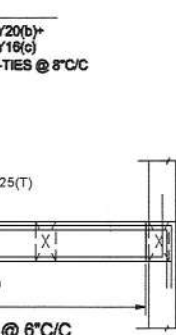
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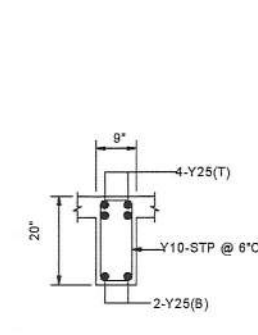
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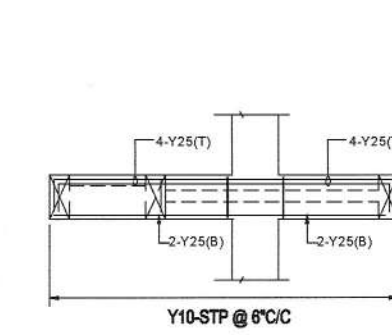
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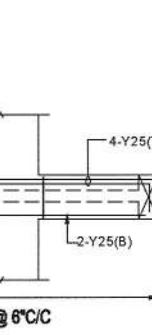
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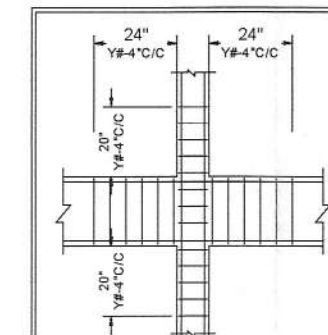
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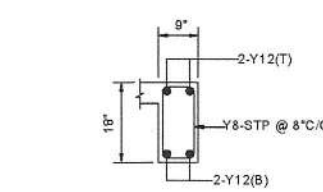
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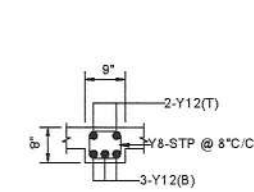
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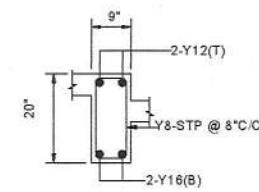
DENOTES 8 OR 10 OR 12MM WHICH EVER MAXIMUM DIA OF THE STRUPTTS BAR USED IN BEAM OR COLUMN



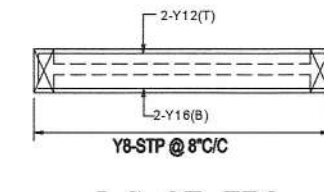
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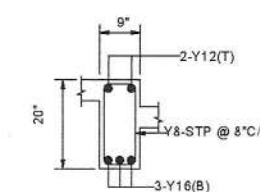
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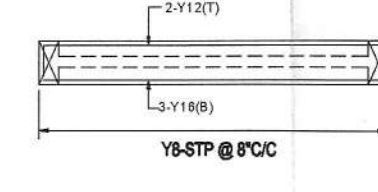
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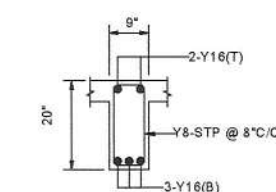
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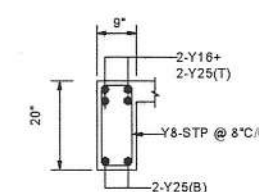
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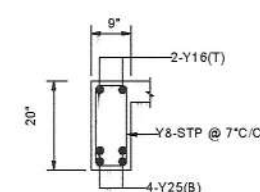
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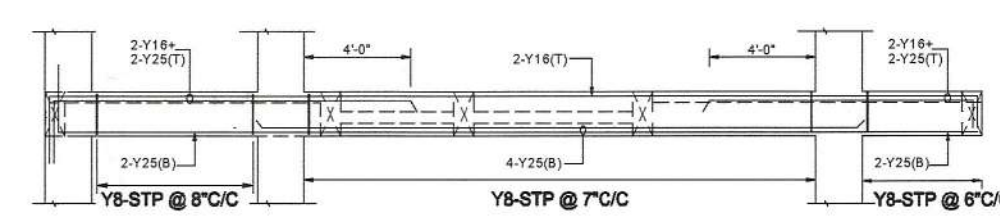
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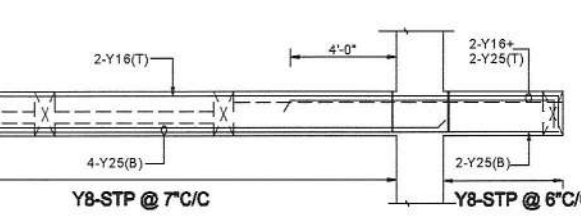
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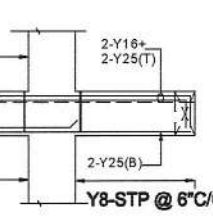
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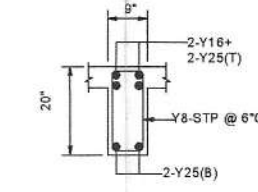
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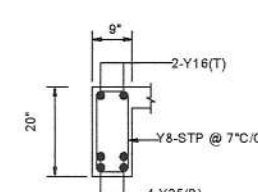
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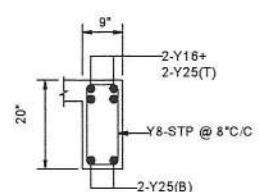
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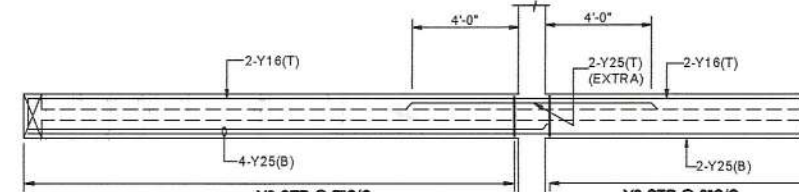
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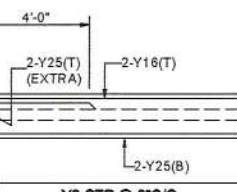
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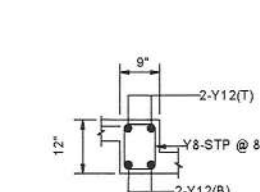
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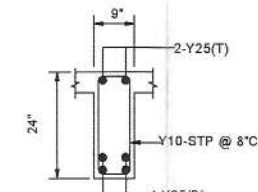
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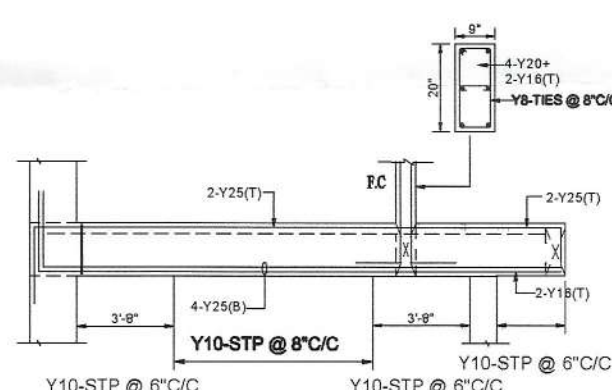
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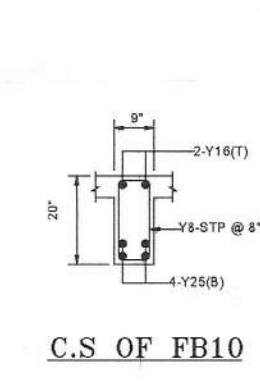
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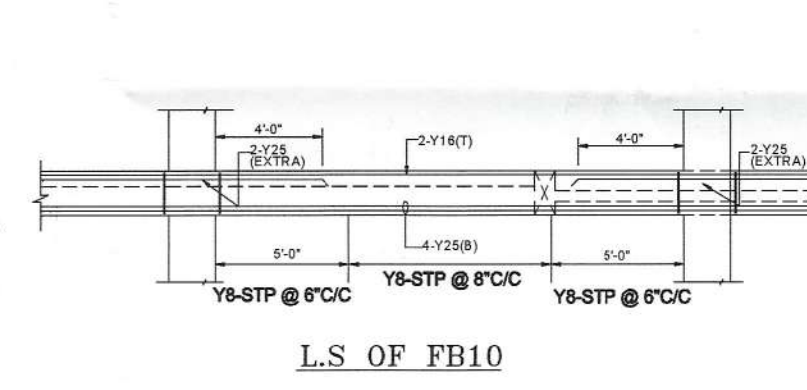
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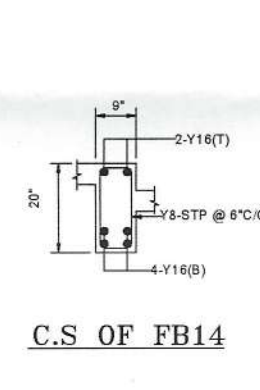
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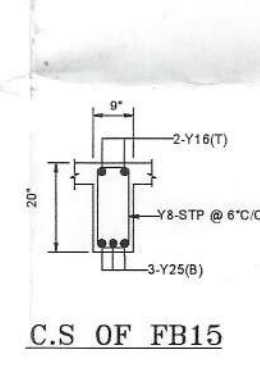
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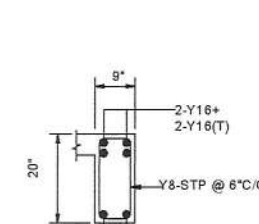
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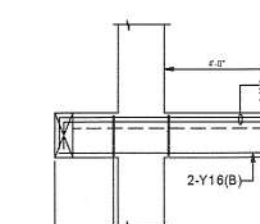
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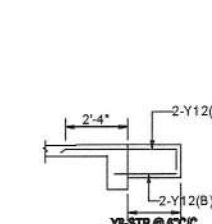
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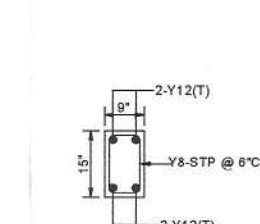
C.S OF CB1



L.S OF CB1



L.S OF DB1



C.S OF DB1

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DATE 05.04.2025

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