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Dt 30.08.2023

Structural stability Certificate

Proposed construction of non-high rise group development consists of 3 blocks with basement of block 3 & combined basement of blocks 4 & 5, block 3 : ground + 5 upper floors (124 dwelling units, height -18.15m) block 4: ground (partly) + 5 upper floors (208 dwelling units, height -18.15m), with clubhouse :ground (partly) + 4 upper floors & swimming pool on the ground floor block 5: ground + 5 upper floors(192 dwellingunits, height-18.15m) totally 524 dwelling units comprised in s.nos. 240/13, 240/15A and 15B of kalavakkam village, thiruporur taluk, chengalpet district.

The design is based on the Architectural plans submitted to Government Authority.

The structure is a reinforced concrete Beam slab with Column & shear walls. The Structure is a RCC Frame structure with shear walls suitably placed and is complying with Bureau of India standard norms and design is safe and suitably for the purpose for which it is intended. The foundation system consists of Isolated footings for column foundation and raft footing for lift shear walls. It has been designed in accordance with IS 456:2000, IS 800:2007 and IS 875:1987. The structure falls under seismic zone III as per the seismic code IS 1893 (part 1) – 2016 and zone factor of 0.16 and is safe and suitable for the purpose for which it is designed.

All the standard stipulations and norms have been followed in designing the building for making the building resistance to earthquake, compliance with structural safety and fire safety requirement.

The building will be sound and stable when used for the purpose which it is designed.

Signature :

Design Manager ,Quadrometry Engineering OPC Pvt. LTD

Name of the Structural Engineer : Rajpurohit Karansingh P.

Structural Engineer Registration No with Stamp :

Coimbatore LPA /SE/GR-I/19/06/063



RAJPUROHIT KARAN SINGH
Structural Engineer Grade - I (SE)
:Reg No: Coimbatore LPA/SE/GR-I/19/06/063

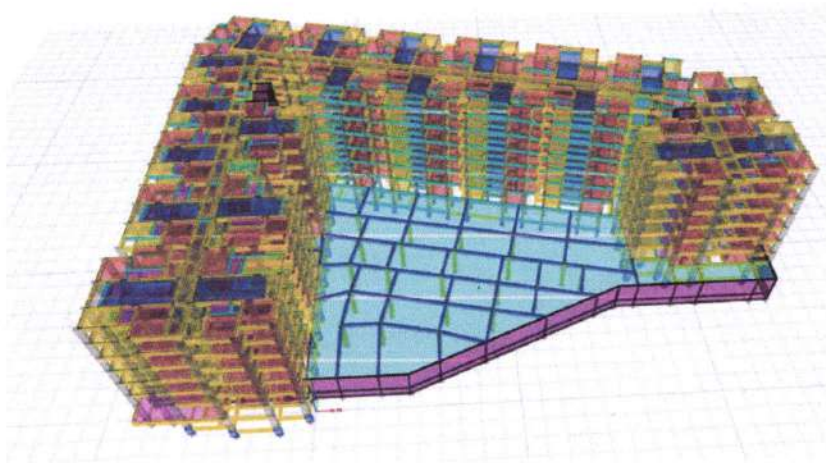
Quadrometry Engineering (OPC) Pvt. Ltd.

#699, 7th Main, HAL 2nd Stage, Indiranagar, Bangalore - 560008
Karnataka, India

STRUCTURAL DESIGN BRIEF REPORT



**M/s CASAGRANDEGRACE PVT.LTD.
RESIDENTIAL BUILDING AT THIRUPUROR
KALAVAKKAM, CHENNAI**



*for
QEPL.
K. S. S. S. S.*

**#610, 17TH B CROSS, INDIRANAGAR II STAGE,
HOYSALA NAGAR, INDIRANAGAR,
BENGALURU-560038**

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1. INTRODUCTION

Quadometry Engineering Pvt. Ltd. has been appointed as Structural Consultants by **M/s Casagrande Grace Pvt. Ltd.** to develop structural schemes and design for proposed low rise residential development at Kalavakkam village, Thiruporur Taluk, Chengalpet District, Chennai.

The objectives of these reports are stated as follows:

- Define the engineering strategy and parameters in relation to the building structure and Architectural concept.
- Identify and record all input requirements, Analysis and design criteria.
- Develop structural scheme compatible with Architectural vision, services requirements and client's needs.
- Prepare structural design that will aim to actual structural durability and integrity.
- Desirable structural performance under characteristic services load. □
- Fast-track construction and cost effectiveness

2. PROJECT DESCRIPTION

The proposed residential development consists of three Blocks 3,4 and 5 along with one Basement as parking, podium/ ground floor. This design basis caters for the Structural Design of Block 3, Block 4 and Block 5 buildings. Podium/ground floor will have landscaping area which will be sunk by 450mm to 600mm with soil filling of 450mm to 600mm over it. Podium/ground floor of Block 4 will have swimming pool having depth of 1200mm. Fire tender movement area is planned over driveway outside the ground floor to access each Blocks on mother earth.

Block-3, Block-4 and Block-5 will have one basement and ground plus 5 stories above. Height of the Blocks is approx. 26m above ground level.

Dimensions of Block-3 are approx. 83m x 61m with following floor heights

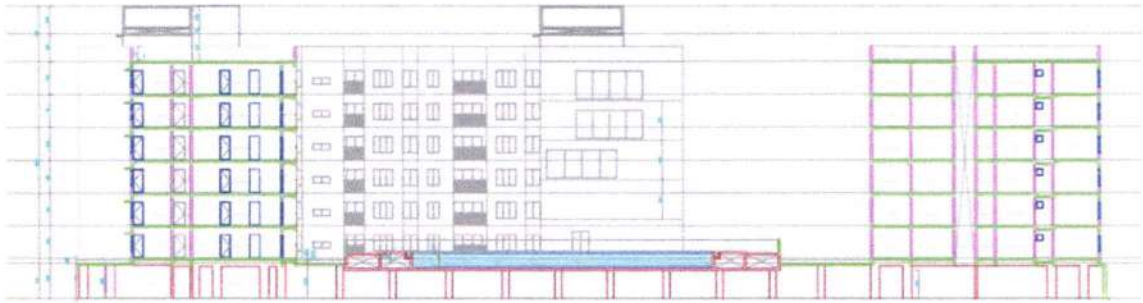
Basement	:	3.6 m
Podium/Ground Floor	:	2.95 m
Typical Floor Second Floor to Terrace	:	2.95m

Dimensions of Block-4 are approx. 92m x 113m with following floor heights

Basement	:	3.6 m
Podium/Ground Floor	:	2.95m
Typical Floor First Floor to Terrace	:	2.95m

Dimensions of Block-5 are approx. 143.5m x 43.32m with following floor heights

Basement	:	3.6 m
Podium/Ground Floor	:	2.95m
Typical Floor First Floor to Terrace	:	2.95m



CONSTRUCTION TYPE

SUBSTRUCTURE

There will not be any basement. The Buildings will be supported on set of Isolated and combined foundations. A grade slab will be provided at the lowest level along with vapour barrier to ensure water tightness on a murum filling compacted to 95% proctor density.

SUPERSTRUCTURE

The superstructure will be constructed in conventional RCC slab system with beams, columns and shear walls. Concrete will be poured in a RCC slab on erection of formwork and fabrication of reinforcement.

3. STRUCTURAL SYSTEM

3.1 FOUNDATION

Based on the proposed structure and sub soil conditions, Geotechnical investigations report recommends shallow foundation system coupled with soil replacement within the foundation trench. As per discussion with Casa Grande Team, Isolated Footings resting on weathered rock is the foundations system of the buildings. The footings will be designed for allowable Bearing Capacity of 300 kN/sq.m at depth of 1m to 2m below basement level. The base slab of UGT and STP will be designed for any upward pressure resulting from Ground water table. It should be noted that 1 m of seasonal variation is considered while fixing the water table level on UGT and STP only.

3.2 SUPERSTRUCTURE

Structural form should contribute to the building character and identity, while being efficient, cost effective and simple to construct. Shear walls are placed in such a way that minimum obstructions are developed to space utility and architect vision.

The building is primarily conventional R.C.C Beam and Slab system supported by beams, columns and shear walls. Floor slabs are considered to act as a rigid diaphragm to transfer the lateral forces to the lateral load resisting elements. Provision is done in design in such a way that services can be laid without any major obstructions and maximum head room is achieved. Kalavakkam is located in Zone-III as classified by IS: 1893-2016. Seismic forces on the

structure will be moderate. Hence RCC Beam - Slab system is considered more suitable structural system as it offers more clear height, provides ease and speed of construction.

4. STRUCTURAL DESIGN STANDARDS AND CODES CONSIDERED

Specific applicable codes and standards will be identified and adopted in the design philosophies as appropriate to the structural elements. The latest editions of the Codes and Standards will be used in designs. All design work shall be based on Indian Standards and Codes with latest revision, with amendments if any, as on date. **Loads**

IS 875(Part 1): 1987:	Dead Loads Unit Weight of Building Material and Stored Material
IS 875(Part 2): 1987:	Imposed Loads
IS 875(Part 3): 2015:	Wind Loads
IS 875(Part 4): 1987:	Snow Loads
IS 875(Part 5): 1987:	Special loads and load combinations
IS 1893:2016:	Criteria for earthquake resistance design of structures.
IS: 13920-2016:	Code of practice for ductile detailing of reinforced concrete structures subjected to seismic forces

Foundations:

IS 1080:1985(2016):	Code of Practice for design and construction of shallow foundations on soils (other than raft, ring and shell).
IS 1904:1986(2015):	Code of Practice for design and construction of foundation in soils general requirements.
IS 2950:1981(2013):	Code of Practice for design and construction of raft foundations
IS 2974:1998(2015/13):	Code of Practice for design and construction of machine foundations (All parts).
IS 8009:1976/1980(2013/15):	Code of Practice for calculations of foundations (All parts).

RCC:

IS 456:2000:	Code of Practice for plain and reinforced concrete
IS 458:1988:	Specification for precast concrete pipes
IS 3370:2009:	Code of Practice for concrete structures for the storage of liquids
IS 4326:2013:	Code of Practice for earthquake resistant design and construction of buildings.
IS 5525:1969(2013):	Recommendation for detailing of reinforced concrete works.
IS 1786:2008:	Specification for high strength deformed steel bars and wires for concrete reinforcement.
IS 10262:2009:	Recommended guidelines for concrete mix design.

IS 1786:2008:	Specification for high strength deformed steel bars and wires for concrete reinforcement.
IS 1893(Part-1) 2016:	Criteria for earthquake resistant design of structures (General provision and buildings).
IS 13920:2016:	Code of practice for ductile detailing of reinforced concrete structures subjected to seismic forces.
IS 1343:2012:	Prestressed Concrete

Structural steel:

IS 800:2007	Code of Practice for general construction in steel.
IS 806:1968(2013):	Code of Practice for use of steel tubes in general building construction
IS 808:1989:	Dimension for Hot rolled steel beam, column channel and angle section.
IS 816:1998:	Code of Practice for use of metal arc welding for general construction in mild steel.
IS 1161:1998:	Steel tubes for Structural Purpose.
IS 4000:1998:	High strength bolts in Steel Structures- Code of Practice
IS:7215 -1995	Tolerances for fabrication of steel structures.

DESIGN LOADS

4.1 Dead Loads

Dead load shall include weight of all structural and Architectural components. Self-weight of the materials shall be calculated based on unit weights given in IS: 875.

Floor finish of 50 to 75 mm thickness

for Parking	:	165 kg/sq.m (1.65 kN/sq.m)
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and 50mm for Residential floors	:	110 kg/sq.m (1.1 kN/sq.m)
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False ceiling / ducting + Sprinklers	:	50 kg/sq.m (0.5 kN/sq.m)
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Self-weight of Plain concrete	:	2400 kg/cu.m (24 kN/cu.m)
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Self-weight of RCC	:	2500 kg/cu.m (25 kN/cu.m)
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Solid block masonry	:	2000 kg/cu.m (20 kN/cu.m)
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Light weight block masonry	:	800 kg/cu.m (8 kN/cu.m)
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(with additional 20mm thick plastering on external surface and 14mm plastering on internal) wall surfaces

Soil (Dry)	:	1800 kg/cu.m (18 kN/cu.m)
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Soil (Wet)	:	2000 kg/cu.m (20 kN/cu.m)
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Water	:	1000 kg/cu.m (10 kN/cu.m)
Glass	:	2800 kg/cu.m (28 kN/cu.m)
Masonry with plastering solid block	:	2000 kg/cu.m (22 kN/cu.m)
Masonry with plastering light block	:	1000 kg/cu.m (10 kN/cu.m)
Filling for sunken portion	:	1200 kg/cu.m (12 kN/cu.m) (Wherever filling is required)
Waterproofing	:	As per actuals
Landscape loads	:	300/450/600 soil filling/finishes as per landscape details
OHT loads above the terraces	:	As per actuals (Shall be referred from MEP dwgs).
Electrical rooms	:	750 kg/sq.m (7.5 kN/sq.m = 5.0 kN/sq.m + 2.5 finishes)
Wall loads	:	As per actuals
Storage rooms	:	750 kg/sq.m (7.5 kN/sq.m)
AHU Rooms if any in club house	:	750 kg/sq.m (7.5 kN/sq.m = 5.0 kN/sq.m + 2.5 finishes)
Lift Machine room	:	1000 kg/ sq.m or as per actual.

4.2 Live Loads

Loads for the building shall be taken as follows as per IS: 875 (Part II),

Corridors, passages, lobbies and Staircases including fire escape	:	3.0 kN/sq.m
Pantry, cafeterias/food courts and Club house floor	:	4.0 kN/sq.m
Living	:	2.0 kN/sq.m
Bedroom	:	2.0 kN/sq.m
Bath and toilet rooms	:	2.0 kN/sq.m
AHU /HUB/Electrical Room	:	7.5 kN/sq.m
Storage	:	5.0 kN/sq.m
Terrace / Balcony	:	3.0 kN/sq.m
Roof (Inaccessible)	:	0.75 kN/sq.m
Single Parking, Ramp	:	5.0 kN/sq.m
Lift machine room	:	10.0 kN/sq.m

Live load reduction is done as per Clause 3.2, IS 875 (Part 2) for the design of columns and footings of the structure.

4.3 Wind Loads

Wind loads have been derived on basis of IS: 875 (Part 3) 2015

Basic wind speed considered V_b : 50 m/s (for Kalavakkam)

Building classification/ topography data

Risk coefficient, k_1 : 1 Table 1, IS 875 (Part 3)

Topography factor, k_3 : 1

Terrain Category : 1 Cl 6.3.2.1, IS 875 (Part 3)

Terrain and Height Multiplier (k_2) will be calculated separately for the building as per Table 2 of IS 875 (Part 3):2015

Importance factor for cyclonic region(k_4) : 1 Cl 6.3.4, IS 875 (Part 3)

Wind pressure calculation (Cl. 7.2, IS 875 (Part 3) 2015)

P_z : $0.6V_z^2$

V_z : $k_1 \times k_2 \times k_3 \times k_4 \times V_b$

P_d : $K_d \times K_a \times K_c \times P_z$

Where K_d : wind directionality factor

K_a : area averaging factor

K_c : combination factor

4.4 Seismic Loads

Seismic design and detailing shall be in accordance with IS 1893 (Part 1):2016 and IS 13920:2016 respectively. Kalavakkam falls under Zone III (moderate) as categorized in IS: 1893-2016, and according to IS 13920, Table 9 Flat Slab with ductile shear wall system is to be provided. All shear walls will be provided with ductile detailing to resist 100% of lateral shear. All beams which are framing into shear walls will be provided with ductile detailing.

Seismic Force is considered in all three directions in accordance with IS 1893 (Part 1):2016.

The seismic design parameters are as follows:

Z : 0.16 (as per Table 3 IS 1893:2016)

I : 1.2 (Building as per Table 8 of IS 1893:2016)

Damping : 5% (as per Cl 7.2.4 of IS 1893:2016)

Soil type I : (for Rock or Hard soils as per Cl 6.4.2 of IS 1893:2016)

R : 5.0 (Refer Table 9; RC building with special moment resisting frame systems (SMRF))

Design Lateral Force: (as per Cl.7.2, IS 1893:2016)

Base shear (V_B) = $A_h \times W$

Where W : Seismic weight of structure as per Cl. 7.4

Ah : Design Horizontal Seismic Coefficient as per Cl. 6.4.2 Time period calculation for Static Base Shear:

T : $0.075 \cdot h^{0.75}$

Where h : Height of building above footing level

Minimum design Lateral Force

Building will be designed for minimum lateral force for 1.1 % of total seismic weight as per Cl.7.2.2 of IS 1893:2016 (Table 7 for Seismic Zone III). For isolated and combined footings with tie beams, SBC will be increased by 50% in earthquake condition for Rock or Hard soils, in accordance with Table 1, Sl. No. (i) of IS 1893 Part 1.

4.5 Temperature Loads on Terrace

As per IS: 875 (Part 5) & IRC: 6-2010 for expansion and contraction due to changes in the temperature of the material of the structure due to daily or seasonal fluctuations in the temperature:

For Kalavakkam, as per IS 875 (Part5)

Maximum air shed temperature 45°C

Minimum air shed temperature 15°C

Mean temperature = $((45) + (15))/2 = 30^{\circ}\text{C}$

Temperature at which the structure is effectively restrained = Mean of maximum and minimum temperature i.e. at 30°C Variation in temperature:

Maximum = $45 - 30 = 15^{\circ}\text{C}$ (TL max)

Minimum = $15 - 30 = -15^{\circ}\text{C}$ (TL min)

This temperature variation shall be applied to terrace.

4.6 Primary Loads and Load Combination

DL	:	Dead Load
LL	:	Live Load
WLX	:	Wind Load in + ve X-Direction
WLY	:	Wind Load in + ve Y-Direction
EQx	:	Seismic Load in X-Direction
EQy	:	Seismic Load in y-Direction
TL max	:	Maximum Temperature load
TL min	:	Minimum Temperature load

Load combinations Used for Strength, Serviceability are tabulated below:

Load Combination	Limit State of Collapse			Limit State of Serviceability		
	<i>DL+SDL</i>	<i>LL</i>	<i>WL/EL/TL</i>	<i>DL+SDL</i>	<i>LL</i>	<i>WL/EL/TL</i>
DL + LL	1.5	1.5	—	1	1	—
DL + WL	1.5 or 0.9*	—	1.5	1	—	1
DL + EL	1.5 or 0.9*	—	1.5	1	—	1
DL + TL	1.5	—	1.5	1	—	1
DL+LL+WL	1.2	1.2	1.2	1	1	1
DL+LL+EL	1.2	1.2	1.2	1	1	1
DL+LL+TL	1.2	1.2	1.2	1	1	1
DL+WL+TL	1.2	—	1.2	1	—	1
DL+EL+TL	1.2	—	1.2	1	—	1
DL+LL+EL+TL	1.2	1.2	1.2	1	1	1

DL - Dead Load; LL - Live Load; TL - Temperature Load; WL - Wind Load; EL - Earthquake Load

Vertical earthquake effect is not considered for the design of structural elements. Wind load and earthquake loads are considered for both x & y directions. Whenever imposed load is combined with earthquake load, the appropriate part of imposed load as specified in IS: 18932016 / Clause 7.3.3 of seismic code will be used both for evaluating earthquake effect and for combined load effects used in such combination. Serviceability loads combinations with suitable live load reduction factor as per IS: 875(Part-2) will be used for foundation design.

5. FIRE RESISITANCE OF ELEMENTS

All structural members will be designed for two-hour fire rating. The requirements for fire resistance will be met by adopting the minimum structural dimensions and clear cover to reinforcement. Minimum dimension of structural element and Nominal cover to reinforcements will be provided as given IS: 456-2000.

6. ENVIRONMENTAL EXPOSURE

The site is situated in Kalavakkam, Chennai which receives moderate rainfall. However, "Severe" exposure is considered for the Substructure in view of the fluctuating ground water table. The minimum nominal cover to the reinforcement will be 45 mm & 50mm for the structural elements like Retaining wall and Footing respectively which are in contact with soil. Also for UG tank and STP walls which are in contact with water, minimum cover shall be

45mm. The members in the Superstructure exposed to weather will be designed for “Moderate” exposure condition. All members in the Super structure within the building will be designed for ‘Mild’ exposure condition as defined in IS 456-2000, Table 16.

7. COVER TO REINFORCEMENT

Minimum clear cover to any reinforcement shall be provided based on the fire requirements and exposure condition whichever is more.

8. MATERIALS

8.1 Concrete

Minimum grade for structural concrete shall be of grade M25. The minimum cement content in concrete for substructure as well as superstructure shall be 300kg/m³ and the maximum free water-cement ratio shall be 0.50. Grade of Concrete for columns and shear walls shall be gradually reduced from M40 at lower floors to M30 at upper levels.

8.2 Reinforcements

High strength deformed steel bar, produced by the thermo-mechanical treatment (TMT) process, of grades Fe 500(D) having elongation more than 14.5 percent as per IS 13920:2016 and conforming to other requirements of IS 1786: 2008 will be used for the reinforcement.

10.3 Structural Steel

All structural steel used for construction shall conform to IS: 2062 with $f_y = 310$ Mpa.

9. ANALYSIS OF STRUCTURE

The analysis will be based on Stiffness matrix method and using the engineering software ETABS 19 version. This is a tried and time tested 3D Structural analysis tool, which includes analysis of plane frames, space frames using truss, beam and shell elements. Generally, all the structures will be idealized as 3D – frames, using line element for column & beams and Shell elements for slabs, shear walls. The structures shall be analysed for individual load cases as described in 6.6 above. Analysis for governing load combinations shall be performed and the performance of the structures in various states of Collapse and Serviceability will be checked accordingly. Manual analysis shall be done for retaining walls, slabs, staircases, water tank etc. The raft and combined footings will be modelled and analysed using software SAFE.

10. DESIGN OF STRUCTURE

The design of the RC and steel structures will be based on the Limit State Method as per IS: 456-2000 and IS: 800-2007 respectively. For liquid retaining concrete structures, the design is based on the Limit State Method, conforming to IS 3370-2009.

The design of RC Columns and beams, footings, slab, staircases and junctions of the structures shall be made using spread sheets developed in-house and/or manual calculations and/or using ETABS software.

11. ACCEPTANCE OF CRITERIA

Deflection Limits:

For Reinforced concrete works: Limiting values of deflection (Short and Long term) as per clause 35.3 of IS: 456-2000 is followed in the design. Limiting value for long term deflection is considered as span/250 and for the long-term deflection after erection of partitions & finishes is considered as span/350 or 20mm whichever less as per IS 456 – 2000.

Cracking Limits:

Limiting values of crack width as per clause 35.3 of IS: 456-2000 is followed in the design.

In general, for slabs and beams, the crack width is limited to 0.3mm.

For basement retaining wall and water retaining structures, crack width is limited to 0.2mm.

Permissible Drift Limits:

Lateral drift = Height/500 (For Wind Loads)

Lateral drift = Height/1000 (at Roof) (For Seismic loads as per Note 4 of Table 9 of IS 1893:2016)

Inter Storey drift = 0.004 x storey height (as per IS 1893:2016)

12. SEPERATION AND EXPANSION JOINTS

Expansion joints will be provided within the building foot print. Pour strips will be provided to allow for initial shrinkage in the concrete. As building dimensions exceed 45m in length (Clause 27.2, IS 456 2000), structure is designed to carry additional stresses as per Clause 2.1, IS 875 (Part 5). The width of expansion joint is calculated on the basis of maximum displacements found for lateral loads between adjoining buildings (Block-1 and Block-2) at podium floors.

13. WATERPROOFING

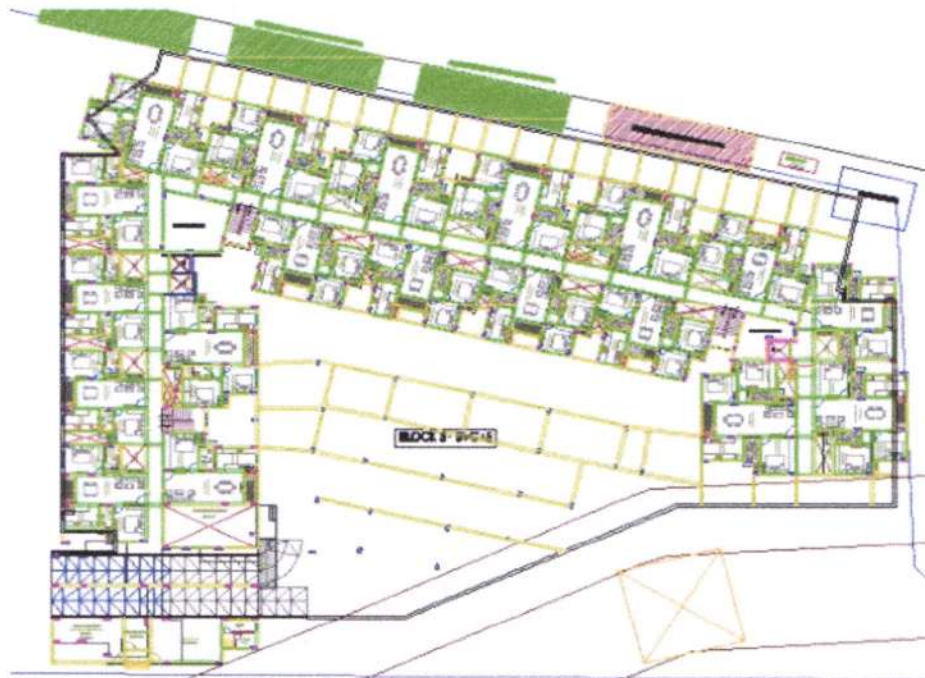
The following Water-Proofing methodology is adopted for various members,

1. Bottom of Foundation, Rafts, Lift pits and Tanks: Self Adhesive HDPE membrane or equivalent.
2. Retaining walls: Sam-Shield XL with protection boards.
3. Podium w/o landscape: Brush bond/Spray apply Polyurethane.
4. Planters/STP/UG (Internal): Brush bond/Spray apply Polyurea.
5. Toilets/Balconies: Dr. fixit - fast flex.
6. Terrace: Brickbat Coba with crystalline mortar or Polyurethane.

ANNEXURE

FRAMING PLANS

PODIUM/GROUND FLOOR FRAMING LAYOUT



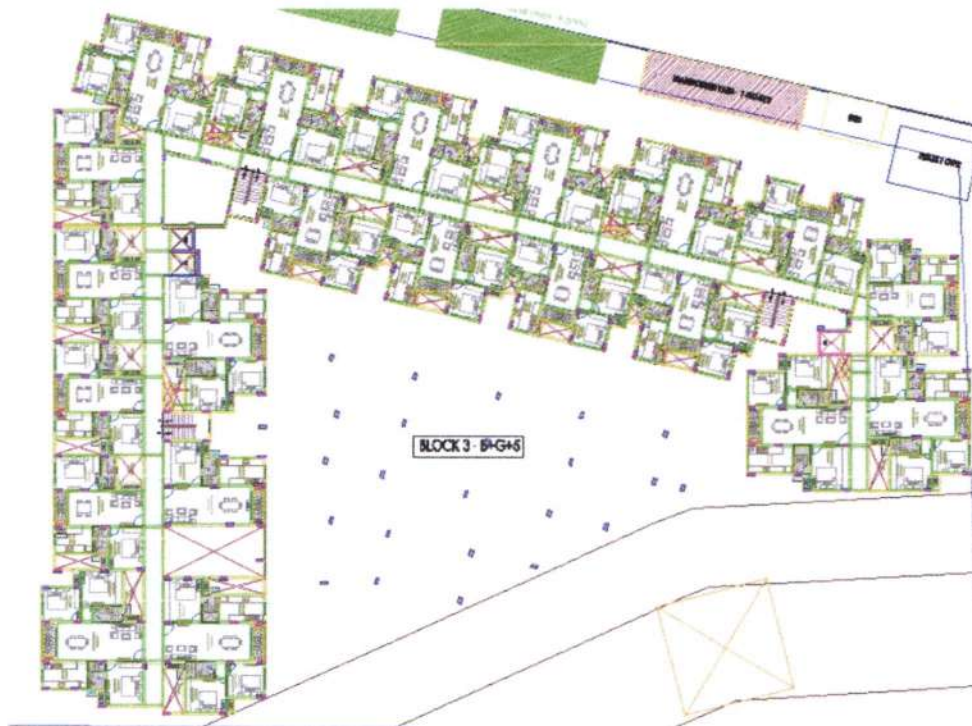
BLOCK-3

BLOCK-1



BLOCK-5

TYPICAL FLOOR FRAMING LAYOUT



BLOCK-3

BLOCK-2



BLOCK-5



our services doesn't just end there



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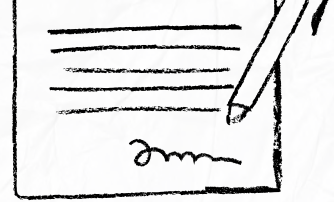
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